



- → Discontinuous Deformation Analysis
- → Contact Dynamics



### OVERVIEW OF DEM SOFTWARES



#### Quasi-static metods

← an <u>equilibrium state</u> is searched for

From an initial approximation of the equilibrium state searched for, the displacements **u** are to be determined taking the system to the equilibrium (assumption: time-independent behaviour, zero accelerations!!!)

$$\mathbf{K} \cdot \Delta \mathbf{u} + \mathbf{f} = \mathbf{0}$$

- → Kishino, 1988
   → Bagi-Bojtár, 1991
   circular, perfectly rigid elemets, deformable contacts

Time-stepping methods  $''\mathbf{M} \cdot \mathbf{a}(t) = \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t))'' \leftarrow a \text{ process in time is searched for } \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t))''$ 

simulate the motion of the system along small, but finite  $\Delta t$  timesteps

#### Explicit timestepping methods:

- $\rightarrow$  UDEC deformable polyhedral elements, deformable contacts
- → BALL-type models, e.g. PFC rigid elements, deformable contacts

#### <u>Implicit timestepping methods:</u>

- $\rightarrow$  DDA (,, Discontinuous Deformation Analysis") deformable polyhedral elements
- → contact dynamics models rigid elements, non-deformable contacts

"DDA": Gen-Hua Shi (1988), Berkeley then many others applied or developed

research softwares!!!

- 1. The elements: The unknowns and the reduced loads
- 2. Contacts
- 3. The equations of motion: Potential energy minimization
- 4. Numerical solution of the equations of motion Applications



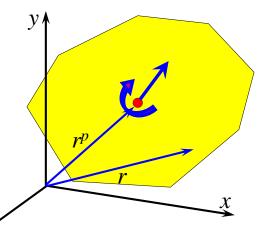


1. The elements:

polyhedral

[Deformable without subdivision]

displacement vector of the *p*-th element:



(reference point;

rigid-body translation and rotation;

the *uniform* strain of the element)

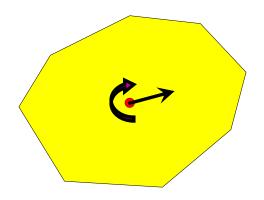
$$\mathbf{u}^p =$$

The translation of another point in the element in e.g. in 2D:
$$\begin{bmatrix} u_{x}(x,y) \\ u_{y}(x,y) \end{bmatrix} = \begin{bmatrix} 1 & 0 & -(y-y^{p}) & (x-x^{p}) & 0 & \frac{(y-y^{p})}{2} \\ 0 & 1 & (x-x^{p}) & 0 & (y-y^{p}) & \frac{(x-x^{p})}{2} \end{bmatrix} \begin{bmatrix} u_{x}^{p} \\ u_{y}^{p} \\ \varphi_{z}^{p} \\ \varepsilon_{x}^{p} \\ \varepsilon_{y}^{p} \\ \gamma_{xy}^{p} \end{bmatrix}$$
in 3D: 
$$\mathbf{u}(x,y,z) = \mathbf{T}(x,y,z) \cdot \mathbf{u}^{p}$$

4/24

**Repetition:** 

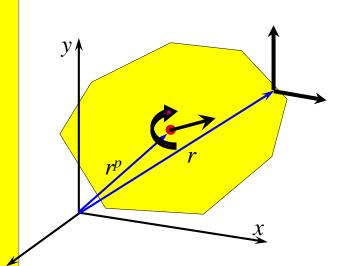
"reduced load" belonging to the *p*-th element:



$$egin{aligned} f_y^p \ f_z^p \ m_x^p \ m_y^p \ V^p \sigma_x^p \ V^p \sigma_z^p \ V^p au_{yz}^p \ V^p au_{zx}^p \ V^p au_{xy}^p \ \end{array}$$



How to reduce a force acting at (x, y) to the reference point of the element:

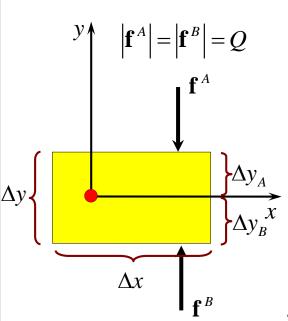


at 
$$(x, y)$$
 to the reference point of the element:
$$\mathbf{f}^{p} = \begin{bmatrix} f_{x}^{p} \\ f_{y}^{p} \\ M_{z}^{p} \\ A^{p}\sigma_{x}^{p} \\ A^{p}\tau_{xy}^{p} \end{bmatrix} \Leftarrow \begin{bmatrix} 1 & 0 \\ 0 & 1 \\ -(y-y^{p}) & (x-x^{p}) \\ (x-x^{p}) & 0 \\ 0 & (y-y^{p}) \\ \frac{(y-y^{p})}{2} & \frac{(x-x^{p})}{2} \end{bmatrix} = \mathbf{T}^{T}(x, y) \mathbf{f}(x, y)$$

$$= \mathbf{T}^T(x, y) \cdot \mathbf{f}(x, y)$$

How to reduce a force acting at (x, y) to the reference point of the element:

e.g. in 2D:



$$\mathbf{f}^{A} = \begin{bmatrix} 0 \\ -Q \end{bmatrix}; \quad \mathbf{f}^{B} = \begin{bmatrix} 0 \\ Q \end{bmatrix}$$

$$y^A - y^P = \Delta y_A \; ; \; y^B - y^P = -\Delta y_B$$

$$\mathbf{f}^{p} = egin{bmatrix} f_{x}^{p} \ f_{y}^{p} \ m_{z}^{p} \ A^{p} \sigma_{x}^{p} \ A^{p} au_{xy}^{p} \end{bmatrix}$$

$$\mathbf{f}^{p} = \begin{bmatrix} f_{x}^{p} \\ f_{y}^{p} \\ m_{z}^{p} \\ A^{p}\sigma_{x}^{p} \\ A^{p}\sigma_{y}^{p} \\ A^{p}\tau_{xy}^{p} \end{bmatrix} \Leftarrow \begin{bmatrix} 1 & 0 \\ 0 & 1 \\ -(y-y^{p}) & (x-x^{p}) \\ (x-x^{p}) & 0 \\ 0 & (y-y^{p}) \\ \frac{(y-y^{p})}{2} & \frac{(x-x^{p})}{2} \end{bmatrix}$$

$$\mathbf{f}^{A} = \begin{bmatrix} 0 \\ -Q \end{bmatrix}; \quad \mathbf{f}^{B} = \begin{bmatrix} 0 \\ Q \end{bmatrix} \qquad \begin{cases} A^{p} \sigma_{x}^{p} = 0 & A^{p} \tau_{xy}^{p} = (x^{A} - x^{p})/2 \cdot f_{y}^{A} + (x^{B} - x^{p})/2 \cdot f_{y}^{B} = 0 \\ A^{p} \sigma_{y}^{p} = (y^{A} - y^{p}) \cdot f_{y}^{A} + (y^{B} - y^{p}) \cdot f_{y}^{B} = 0 \end{cases}$$

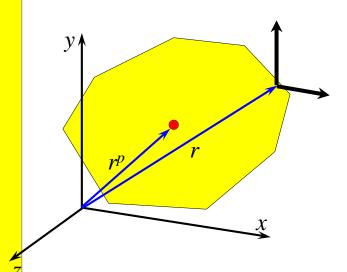
$$= \Delta y_A \cdot (-Q) + (-\Delta y_B) \cdot Q = -(\Delta y_A + \Delta y_B) \cdot Q = -\Delta y \cdot Q$$

$$\Delta y \cdot \Delta x \cdot \sigma_y^p = -\Delta y \cdot Q$$

$$\sigma_y^p = -\frac{Q}{\Delta x}$$
7/24

$$\sigma \cdot \Delta x \cdot \sigma_y^p = -\Delta y \cdot Q \qquad \sigma_y^p = -\frac{Q}{\Delta x}$$

How to reduce a force acting at (x, y) to the reference point of the element:



the reduced force in 3D:  $\mathbf{T}^{T}(x, y, z) \cdot \mathbf{f}(x, y, z)$ 

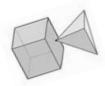
Remark: Higher-order polynomials also possible! [strains/stresses] (e.g: M. MacLaughlin)



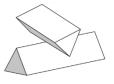
(material point) with (material point)

 $\underline{\text{in } 2D:}$  Node – to – Edge contacts

<u>in 3D:</u> Node – to – Face contacts:



Edge - to - Edge contacts:



 $\rightarrow$  , first entrance position"

 $\Rightarrow$  contact deformation:  $\Delta u_N$ ;  $\Delta u_T$  normal & tangential, perhaps sliding

→ direction of the contact:

the normal vector of the face ???? for edge-to-edge contact

Mechanical model:

→ originally: infinitely rigid contacts, Coulomb-friction

→ recent codes: deformable contacts included

+ other friction conditions, cohesion etc.

<u>Remark:</u> infinitely rigid contact: "penalty function":  $F_N = k_N \Delta u_N$ ;  $dF_T = k_T d(\Delta u_T)$ 

 $\equiv$  linearly elastic in normal and in tangential directions<sub>9/24</sub>

↓ more exactly: "Hamilton principle"

3. The equations of motion: "Potential energy" stationarity principle

"Potential" of the system:

$$\frac{\partial \Pi}{\partial u_i^p} = 0 \quad \text{for all } p, i$$

Π = Π<sup>blocks</sup> + Π<sup>contacts</sup>
 deformed springs
 external pot.
 strain energy
 inertial forces
 velocity-proportional damping
 initial stress
 prescribed displacement history

#### 4. Numerical solution of the equations of motion:

 $(\underline{t_i}, \underline{t_{i+1}})$  time interval:

at  $t_i$ : known  $\mathbf{u}_i$ ,  $\mathbf{v}_i$ ,  $\mathbf{f}(t_i, \mathbf{u}_i, \mathbf{v}_i)$ ; satisfy the eqs. of motion

Find  $\mathbf{u}_{i+1}$ ,  $\mathbf{v}_{i+1}$ ,  $\mathbf{a}_{i+1}$  so that the eqs of motion would be satisfied at  $t_{i+1}$ 

$$\mathbf{r}(t_{i+1}, \mathbf{u}_{i+1}, \mathbf{v}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}, \mathbf{v}_{i+1}) - \mathbf{M} \cdot \mathbf{a}_{i+1} = 0$$

Remember:

Newmark's 
$$\beta$$
-method: 
$$\mathbf{u}_{i+1} = \mathbf{u}_i + \Delta t \cdot \mathbf{v}_i + \frac{\Delta t^2}{2} \left[ (1 - 2\beta) \mathbf{a}_i + 2\beta \cdot \mathbf{a}_{i+1} \right]$$
[stability:  $2\beta \ge \gamma \ge \frac{1}{2}$ ] 
$$\mathbf{v}_{i+1} := \mathbf{v}_i + (1 - \gamma) \cdot \Delta t \cdot \mathbf{a}_i + \gamma \cdot \Delta t \cdot \mathbf{a}_{i+1}$$

DDA:

Newmark's 
$$\beta$$
-method, with  $\beta = 1/2$ ;  $\gamma = 1$ :
$$\mathbf{u}_{i+1} = \mathbf{u}_i + \Delta t \cdot \mathbf{v}_i + \frac{\Delta t^2}{2} \mathbf{a}_{i+1}$$

$$\mathbf{v}_{i+1} := \mathbf{v}_i + \Delta t \cdot \mathbf{a}_{i+1}$$
let
$$\mathbf{a}_{i+1} = \frac{1}{\Delta t^2/2} (\Delta \mathbf{u}_{i+1} - \Delta t \cdot \mathbf{v}_i)$$

$$\mathbf{v}_{i+1} = \mathbf{v}_i + \Delta t \cdot \mathbf{a}_{i+1} = \mathbf{v}_i + \frac{2}{\Delta t} (\Delta \mathbf{u}_{i+1} - \Delta t \cdot \mathbf{v}_i) = \frac{2}{\Delta t} \Delta \mathbf{u}_{i+1} - \mathbf{v}_i$$

$$11/24$$

#### 4. Numerical solution of the equations of motion :

$$\mathbf{M} \cdot \mathbf{a}(t) = \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t)) \implies \mathbf{0} = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}, \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$$

Determine  $\Delta \mathbf{u}_{i+1}$ , so that the residual

$$\mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}), \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$$

would be sufficiently close to zero!

#### Newton-Raphson:

the Jacobian of the residual:  $\mathcal{K}(t, \Delta \mathbf{u}) = \frac{d\mathbf{r}(t, \Delta \mathbf{u})}{d\Delta \mathbf{u}}$ 

this matrix can be compiled from elementary calculations at  $t_i$ :

← contains the stiffness matrix

← contains the inertia, contact forces, geometric characteristics etc.

the residual can also be compiled from elementary calculations at  $t_i$ :  $\leftarrow$  contains the external forces, inertia effects,
prescribed displacements, damping etc.

#### 4. Numerical solution of the equations of motion :

$$\mathbf{M} \cdot \mathbf{a}(t) = \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t)) \implies \mathbf{0} = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}, \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$$

$$\mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}), \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$$

$$\mathcal{K}(t, \Delta \mathbf{u}) = \frac{d\mathbf{r}(t, \Delta \mathbf{u})}{d\Delta \mathbf{u}}$$

#### Analysis of a time interval:

initial estimation for  $\Delta \mathbf{u}_{i+1}$ :  $\Delta \mathbf{u}_{i+1}^{(0)} := \mathbf{0}$ 

k+1-th estimation for  $\Delta \mathbf{u}_{i+1} : \Delta \mathbf{u}_{i+1}^{(k+1)} := \Delta \mathbf{u}_{i+1}^{(k)} - \mathcal{K}(t_{i+1}, \Delta \mathbf{u}_{i+1}^{(k)})^{-1} \cdot \mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}^{(k)})$ 

then continue until  $\left|\mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}^{k+1})\right|$  becomes sufficiently small

"Open – close iterations": at the end of  $\Delta t$ : **check** the topology and the forces;

- → modify the topology if necessary (e.g. new contacts, sliding, contact loss)
- $\rightarrow$  with the new topology, **repeat:** Newton-Raphson to find another  $\Delta \mathbf{u}_{i+1}$

if acceptable topology not found: decrease timestep  $\Delta t$  to 1/3 of its previous length

Comparison to UDEC:

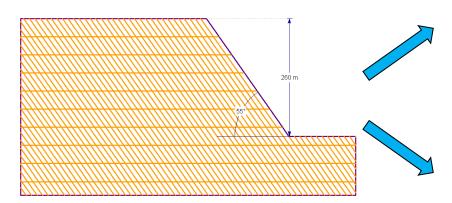
#### Main differences from UDEC:

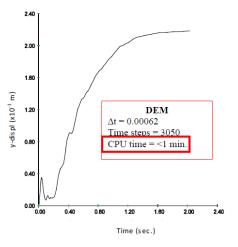
- $\rightarrow$  basic unknowns: also the components of  $\varepsilon$ ;
- → uniform stress and strain field inside the elements;
- → numerical integration: implicit
- $\rightarrow$  stiffness matrix included  $\Rightarrow$  artificial damping not necessary
- <u>advantages to UDEC:</u> → implicit ⇒ numerical stability;
   fast convergence if topology does not change no artificial damping required
- disadvantages: no commercial software ⇒ inconvenient
   (several research codes; e.g. ask from Gen-Hua Shi)
   too simple mechanics of the elements and of the contacts
   large storage requirements & longer computations
   open-close iterations: convergence is not ensured if topology changes

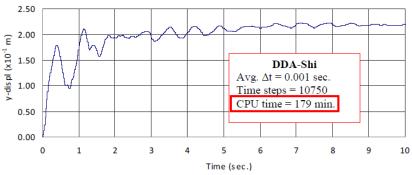
Remark 1:

Comparison to UDEC:

M.S. Kahn (2010)





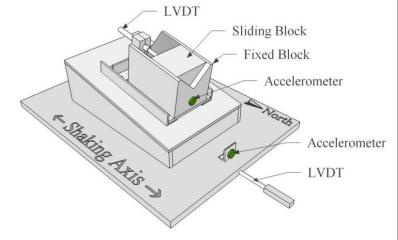


NOT EFFICIENT IN CASES IF SIGNIFICANT TOPOLOGY MODIFICTIONS OCCUR !!!

Remark 2: <u>Importance of experimentally verified material parameters</u>

e.g. D.B. Mazor (2011): in DDA-3D, Aim: Simulate shaking table experiments What is the proper friction angle?





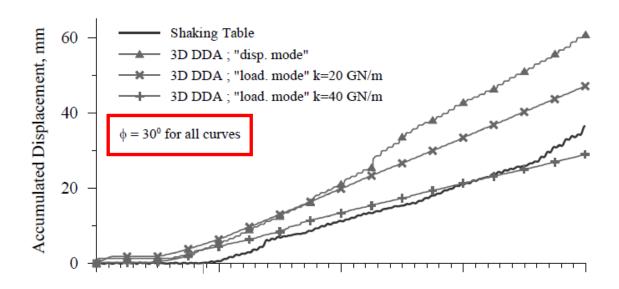
measurement of friction angle:  $\phi = 36^{\circ}$ 

Remark 2: <u>Importance of experimentally verified material parameters</u>

e.g. D.B. Mazor (2011): in DDA-3D,

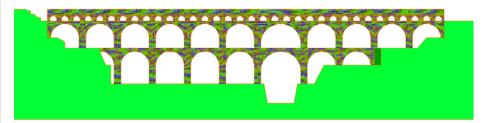
Aim: Simulate shaking table experiments

What is the proper friction angle?

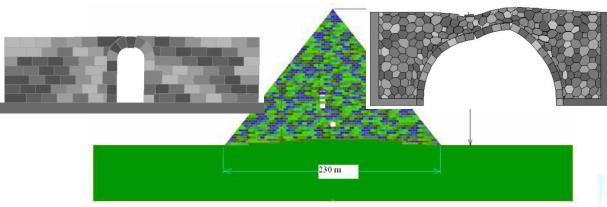


- ⇒ experience: "friction degradation" during dynamic sliding
- ⇒ importance of contact penalty stiffness

#### **Applications**



arches, arch bridges



masonry structures

e.g. Fractured rock for earthquake and thermal effects

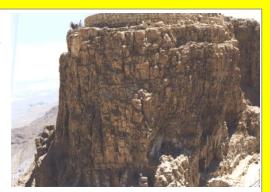


underground excavations



### **DISCONTINUOUS DEFORMATION**

Y.H.Hatzor, 1999-2004: Masada-hill, Israel (King Herod's Palace)



Herod the Great built his fortress a hundred kilometers away from Jerusalem, partly as a retreat, but more as a place of refuge for himself.

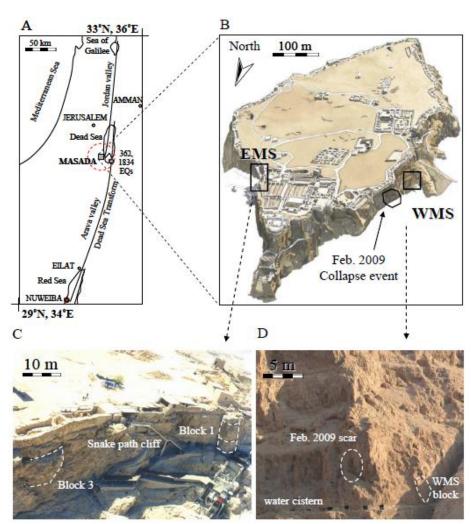
It happened at Masada seventy years after Herod's death that the Jewish Revolt against the Roman occupation was well underway. As the story goes, a last brave group of Jewish rebels fled Jerusalem to Masada, and withstood a two-year long siege by a 15,000-strong Roman Army. The crafty Romans finally built a ramp up the mountain slope.

On their last night in Masada before the Romans broke through, the 967 rebels opted for mass suicide rather than surrender.

"Masada must not fall again"

Y.H.Hatzor, 1999-2004: Masada-hill, Israel (King Herod's Palace)





Y.H.Hatzor, 1999-2004:

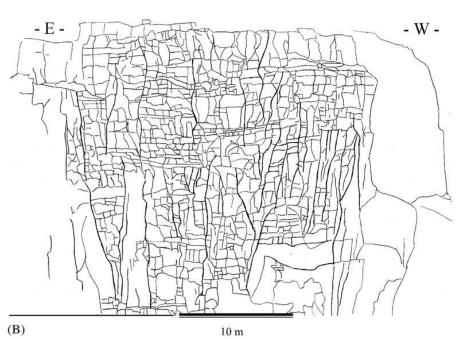
Masada-hill, Israel (King Herod's Palace)

experience: huge blocks released, or already fallen out

the aim of the analysis: is it stable for an earthquake? "Masada must not fall again"

– DDA model, earthquake simulation;





Y.H.Hatzor, 1999-2004:

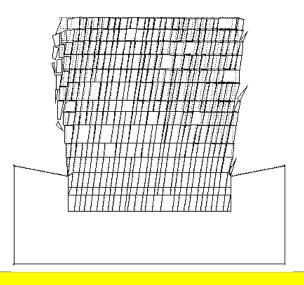
Masada-hill, Israel (King Herod's Palace)

experience: huge blocks released, or already fallen out

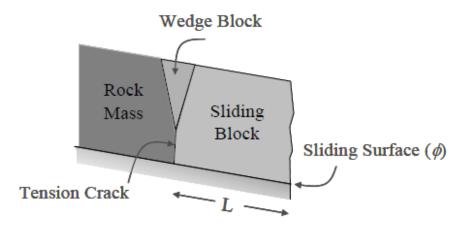
the aim of the analysis: is it stable for an earthquake? "Masada must not fall again"

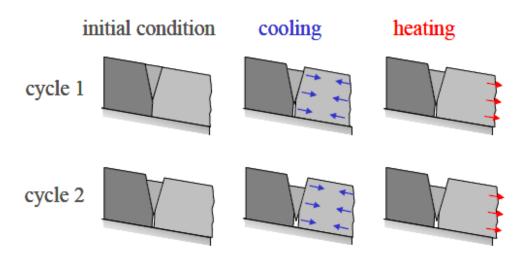
- DDA model, earthquake simulation;
- minor earthquake: considerable damages can be expected;
- 7,1 earthquake (1995, Sinai-desert): huge blocks would fall out
- ⇒ proposals for strengthening

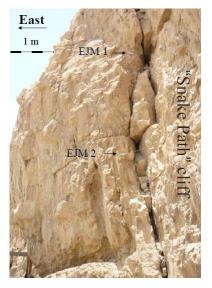


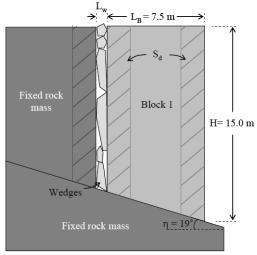












### **QUESTIONS**



- 1. How the displacement vector of an element looks like in DDA? (i.e., what are the degrees of freedom of an element in DDA?)
- 2. Introduce the mechanical model of the contacts in DDA.
- 3. How does the equation of motion looks like in DDA? Write it, and tell the name of the terms in it.
- 4. What kind of time integration method is used in DDA? What values of the numerical control parameters is chosen in it, and what is the mechanical meaning of this choice?
- 5. Explain the meaning of the expression "open-close iteration" and how it is used in DDA.
- 6. Tell five differences between DDA and 3DEC.