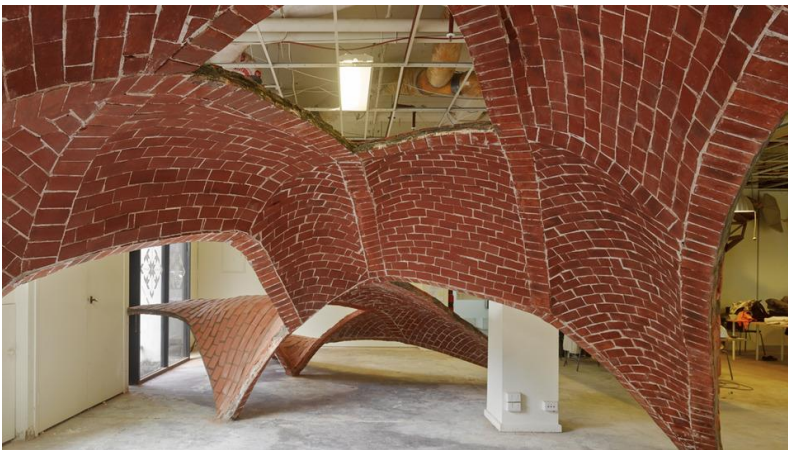




SHELL THEORIES



Citation:

K. Bagi (2024): **Mechanics of Masonry Structures**. Course handouts, Department of Structural Mechanics, Budapest University of Technology and Economics

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In case of any question or problem, do not hesitate to contact Prof. K. Bagi, kbagi.bme@gmail.com .

THIS LECTURE:

SHELL THEORIES

Definition of shells and the shell element

The most important shell theories

Membranes; soap films

Kirchhoff-Love shells („classical shells”, „shell bending”)

Reissner-Mindlin shells („linear shear deformation theory”)

Third-order shear deformation theory

Membrane solution: Examples

Repetition from Maths: Principal curvatures at a surface point

Membrane solution for spherical domes

Membrane solution for fan vaults

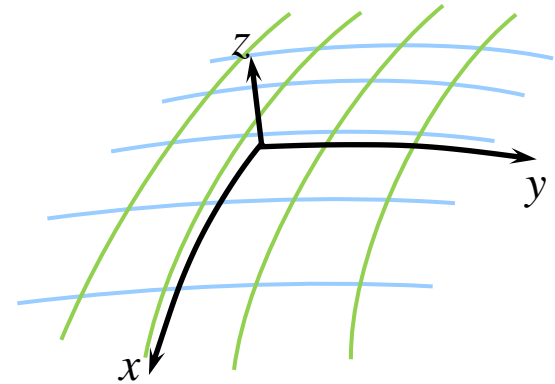
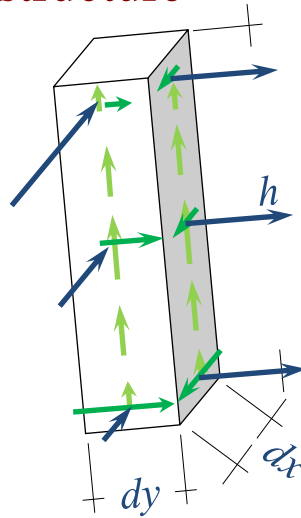
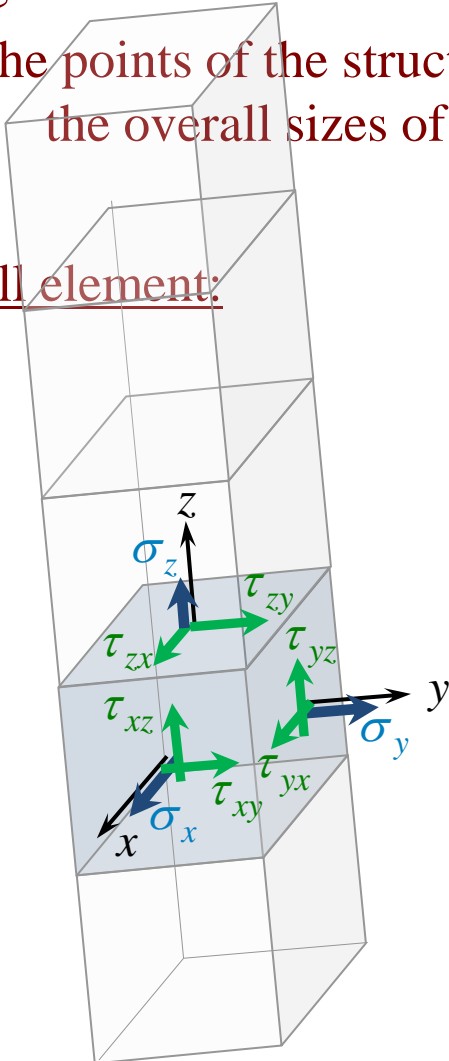
Questions

Definition of shells

→ given reference surface;

→ the points of the structure are within a small distance from it, compared to the overall sizes of the structure

Shell element:



Note: top and bottom elementary volume:

$$\tau_{zx} = 0 ; \tau_{zy} = 0 \quad (\text{free surface})$$

⇒ at the intrados and extrados:

$$\tau_{xz} = 0 ; \tau_{yz} = 0$$

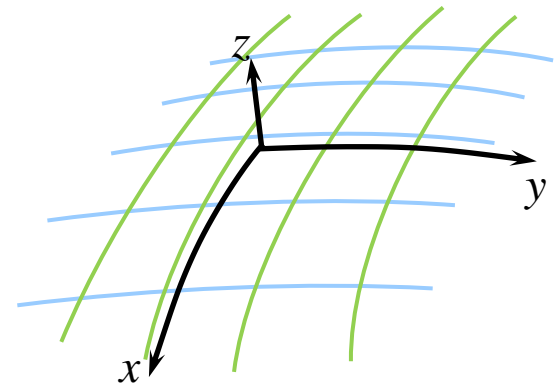
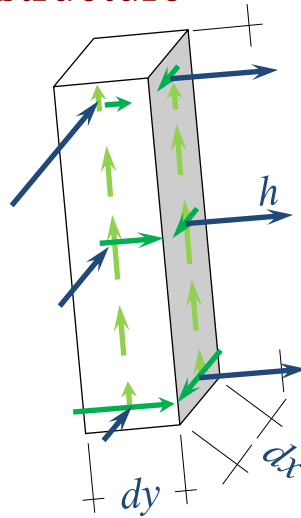
Definition of shells

- given reference surface;
- the points of the structure are within a small distance from it, compared to the overall sizes of the structure

Shell element:

The most common models depending on the thickness:

- membrane
- Kirchhoff – Love shell („classical” shell theory)
- Mindlin – Reissner shell
(„linear shear deformation theory”)
- third-order shear deformation model
- thick shells: 3D continuum or DEM



increasing h

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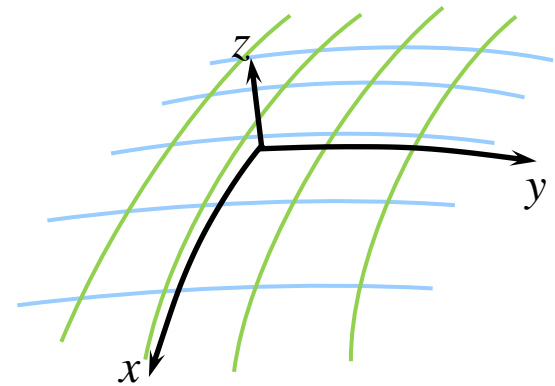
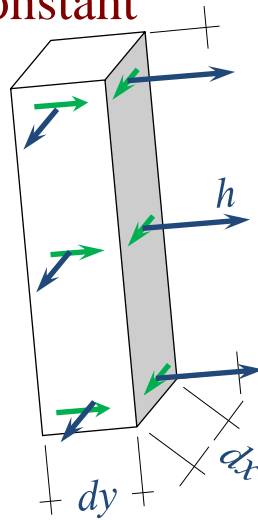
The most important shell theories

Membranes:

→ stresses:

along h : σ_x , σ_y and τ_{xy} are constant

σ_z , τ_{xz} and τ_{yz} are zero



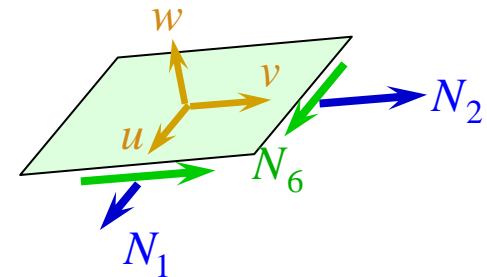
→ displacements:

translations of the points of the reference surface:

$$u(x,y), v(x,y), w(x,y)$$

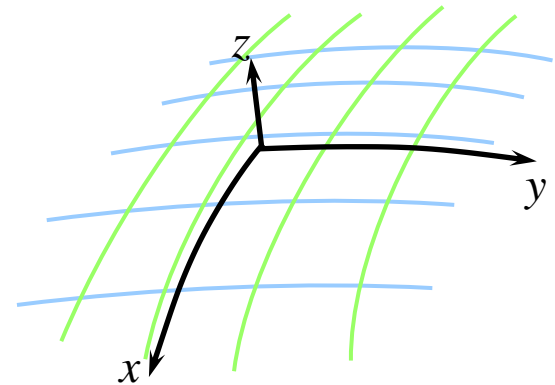
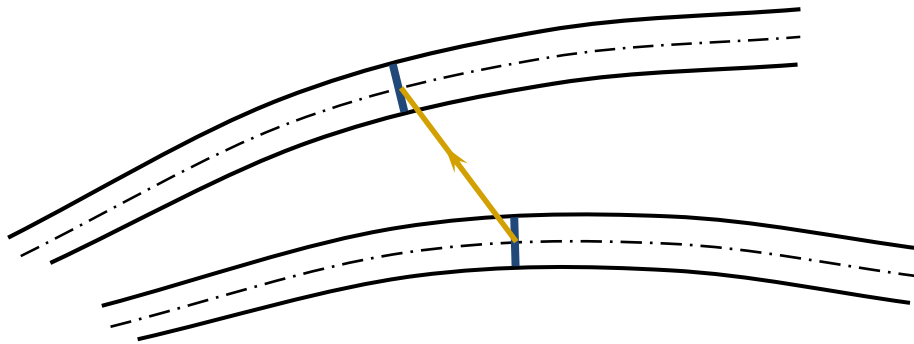
translations of the other points:

$$u_1(x,y,z)=u(x,y), u_2(x,y,z)=v(x,y), u_3(x,y,z)=w(x,y)$$



The most important shell theories

Membranes:



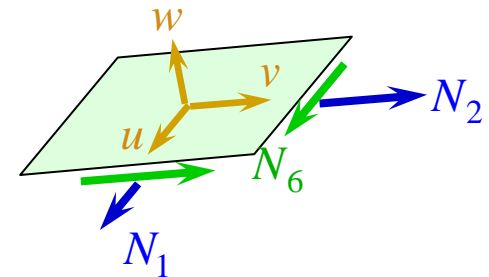
→ displacements :

translations of the points of the reference surface :

$$u(x,y), v(x,y), w(x,y)$$

translations of the other points:

$$u_1(x,y,z)=u(x,y), u_2(x,y,z)=v(x,y), u_3(x,y,z)=w(x,y)$$



The most important shell theories

Membranes:

Special case: „SOAP FILMS”

if the material cannot carry shear:

→ stresses:

along h : σ_x , σ_y , τ_{xy} : constant:

σ_x and σ_y are equal, $\tau_{xy} = 0$

σ_z , τ_{xz} , τ_{yz} zero

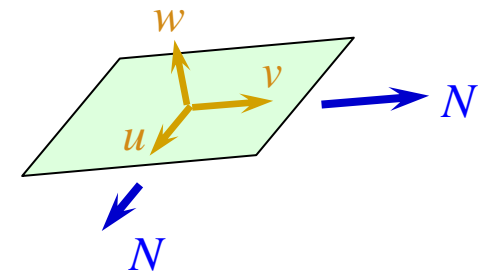
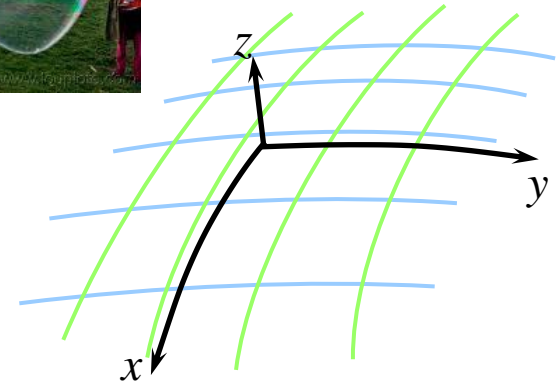
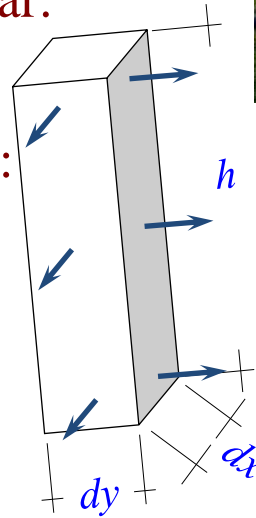
→ displacements:

translations of the points of the reference surface:

$u(x,y)$, $v(x,y)$, $w(x,y)$

translations of the other points :

$u_1(x,y,z)=u(x,y)$, $u_2(x,y,z)=v(x,y)$, $u_3(x,y,z)=w(x,y)$



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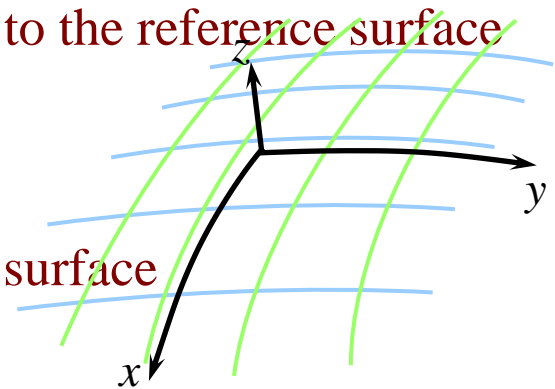
Kirchhoff – Love – shells: „classical shells”, „shell bending”

Kirchhoff-hypothesis:

the planar cross sections being perpendicular to the reference surface
will, after deformation, remain

→ planar, and

→ perpendicular to the (deformed) reference surface



Basic assumptions of Love:

- (1) the shell is thin
- (2) translations and rotations are small
- (3) straight material lines being orthogonal to the reference surface
will remain orthogonal also after deformation
- (4) crosswise shear stresses are negligible

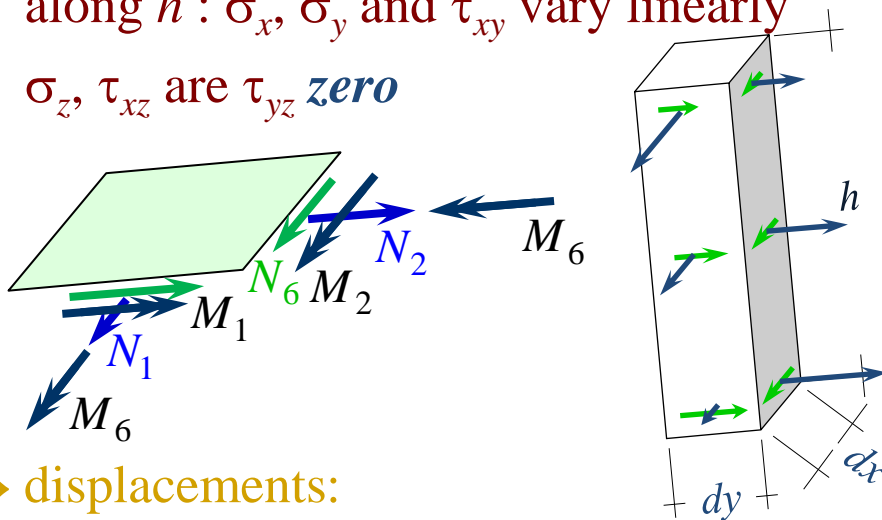
The most important shell theories

Kirchhoff – Love – shells: „classical shells”, „shell bending”

→ stresses:

along h : σ_x , σ_y and τ_{xy} vary linearly

σ_z , τ_{xz} are τ_{yz} **zero**



→ displacements:

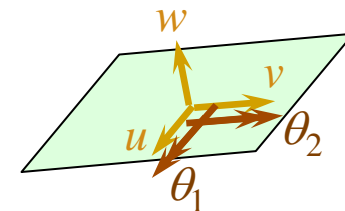
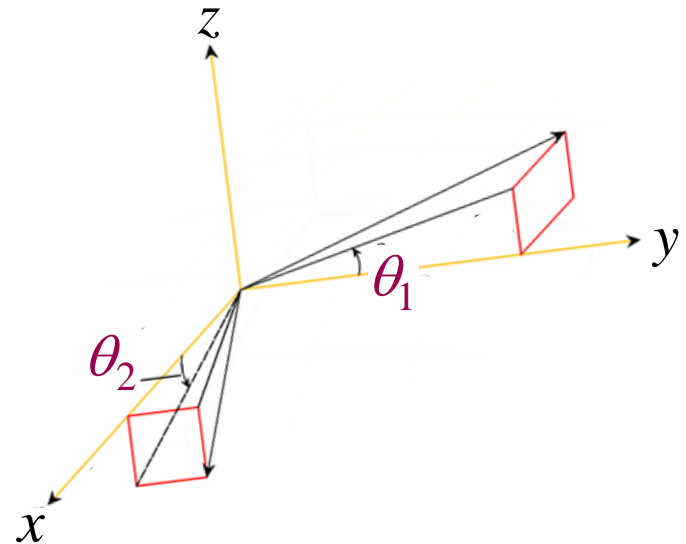
points of the reference surface:

$$u(x,y), v(x,y), w(x,y), \Rightarrow \theta_1(x,y), \theta_2(x,y)$$

translations of other points: $u_1(x, y, z) = u(x, y) + z \cdot \theta_2$

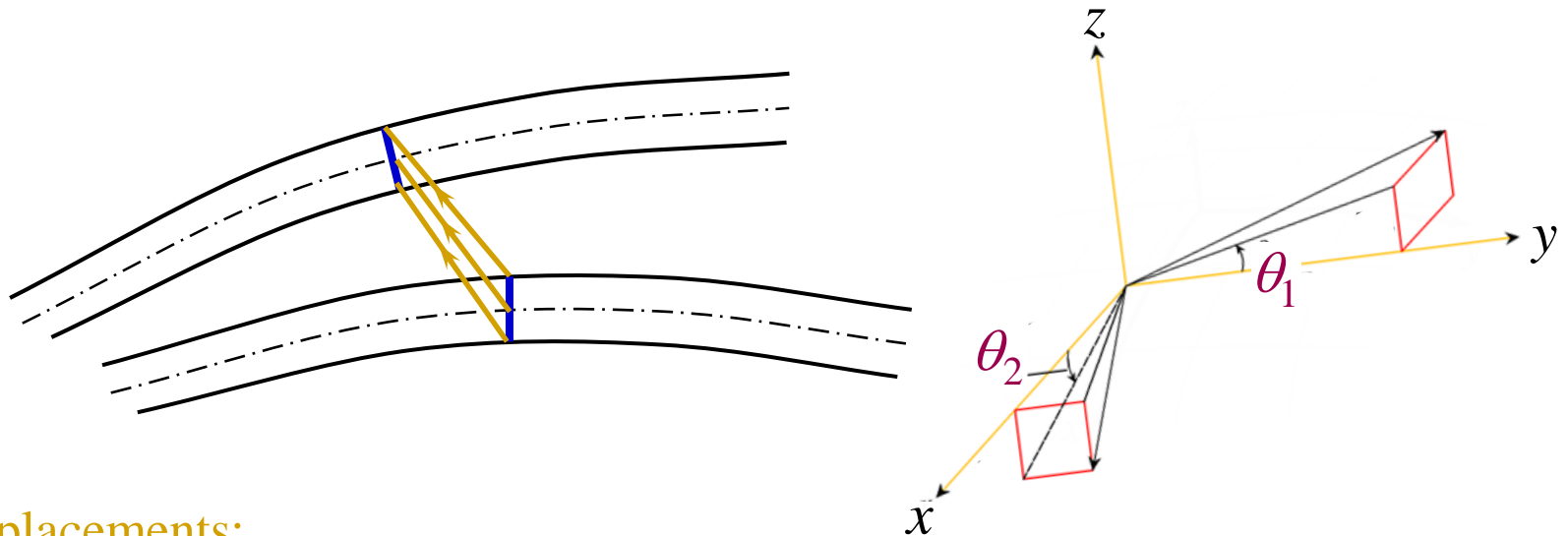
$$u_2(x, y, z) = v(x, y) - z \cdot \theta_1$$

$$u_3(x, y, z) = w(x, y)$$



The most important shell theories

Kirchhoff – Love – shells: „classical shells”, „shell bending”



→ displacements:

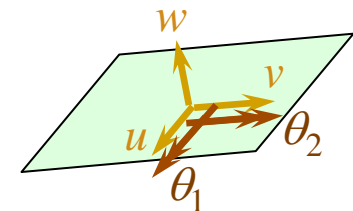
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The most important shell theories

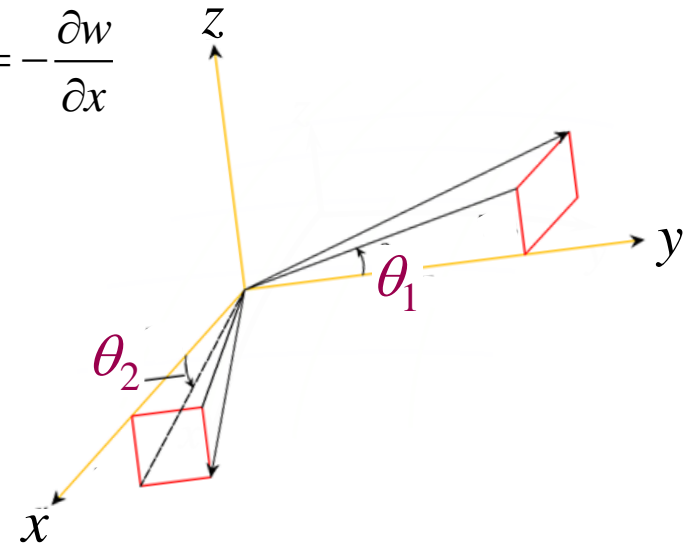
Kirchhoff – Love – shells: „classical shells”, „shell bending”

Remark:

for planar reference surface: $\theta_1 = \frac{\partial w}{\partial y}$; $\theta_2 = -\frac{\partial w}{\partial x}$

for curved reference surface:

θ_1 and θ_2 can uniquely be determined
from the curvatures and from $\frac{\partial w}{\partial y}$; $\frac{\partial w}{\partial x}$



→ displacements:

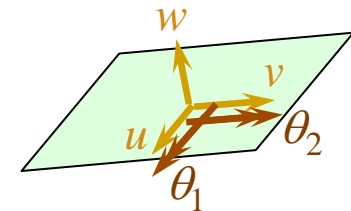
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Reissner – Mindlin – shells: „linear shear deformation theory”

planar sections perpendicular to the reference surface will, after deformation,

→ remain planar (linear warping function), but

→ not perpendicular to the (deformed) ref. surface

crosswise shear effects *are* taken into account!

→ displacements:

points of the reference surface:

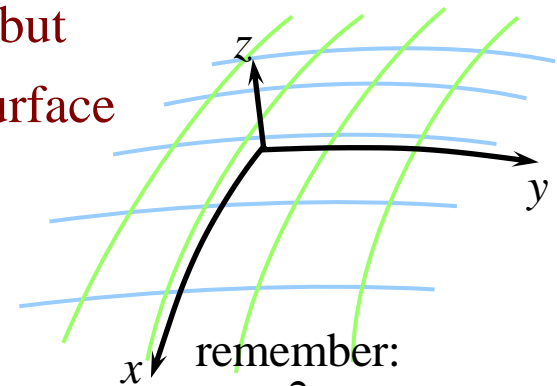
$$u(x,y), v(x,y), w(x,y), \Rightarrow \theta_1(x,y), \theta_2(x,y);$$

translations of other points: $g(z) := z$

$$u_1(x, y, z) = u(x, y) + z \cdot \theta_2 + g(z) \cdot \gamma_5$$

$$u_2(x, y, z) = v(x, y) - z \cdot \theta_1 + g(z) \cdot \gamma_4$$

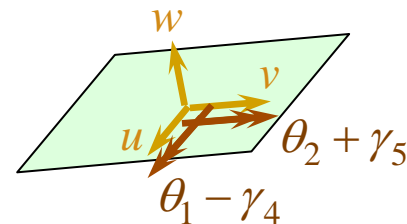
$$u_3(x, y, z) = w(x, y)$$



remember:

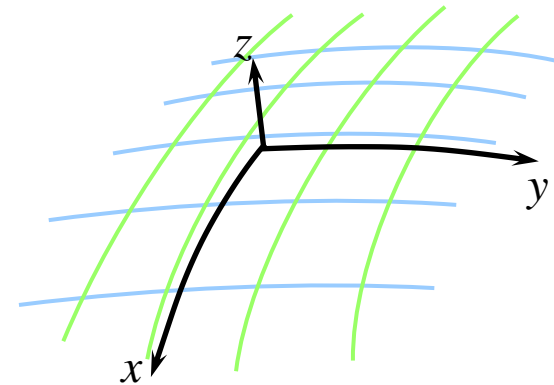
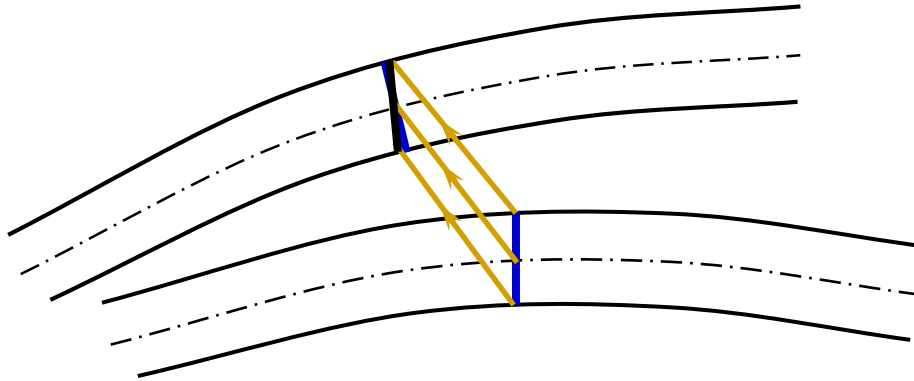
$$\theta_1 = \frac{\partial w}{\partial y} + \dots; \theta_2 = -\frac{\partial w}{\partial x} + \dots$$

$$\gamma_4(x,y), \gamma_5(x,y)$$



The most important shell theories

Reissner – Mindlin – shells: „linear shear deformation theory”



→ displacements:

points of the reference surface:

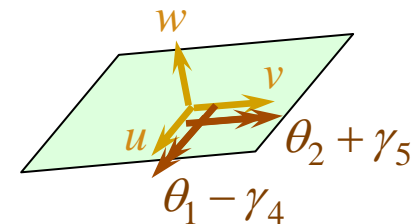
$$u(x,y), v(x,y), w(x,y), \Rightarrow \theta_1(x,y), \theta_2(x,y); \quad \gamma_4(x,y), \gamma_5(x,y)$$

translations of other points: $g(z) := z$

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$$u_2(x, y, z) = v(x, y) - z \cdot \theta_1 + g(z) \cdot \gamma_4$$

$$u_3(x, y, z) = w(x, y)$$



The most important shell theories

Reissner – Mindlin – shells: „linear shear deformation theory”

planar sections perpendicular to the reference surface will, after deformation

→ remain planar (linear warping function), but

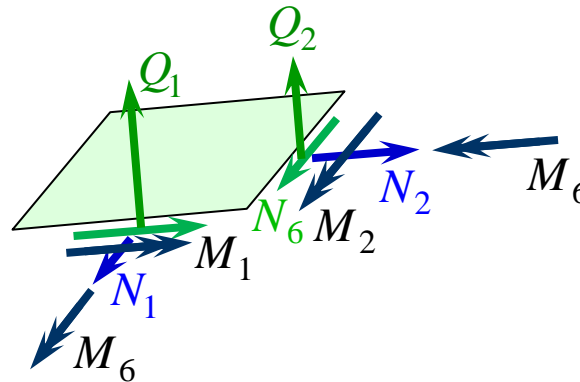
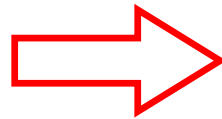
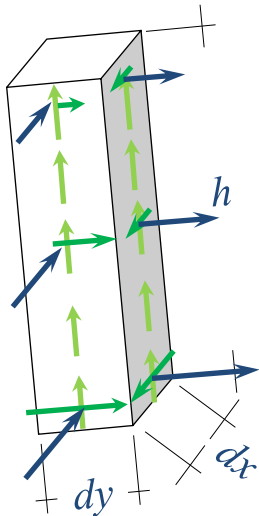
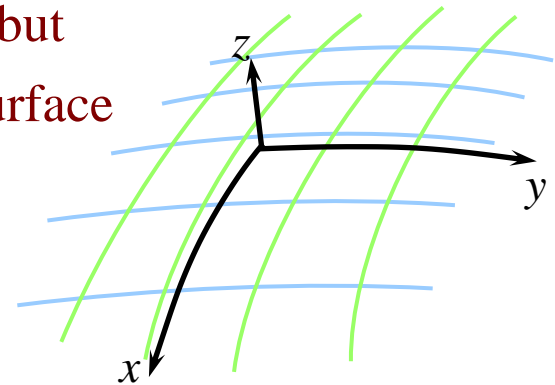
→ not perpendicular to the (deformed) ref. surface

crosswise shear effects taken into account!

→ stresses: along h : σ_x , σ_y and τ_{xy} vary linearly

σ_z is zero

τ_{xz} and τ_{yz} : linear for curved surface (constant for planar shell)



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Third-order shear deformation theory:

$$g(z) := z - \frac{4z^3}{3h^2}$$

→ stresses:

τ_{xz} and τ_{yz} : able to be zero on the free surfaces 😊

→ displacements:

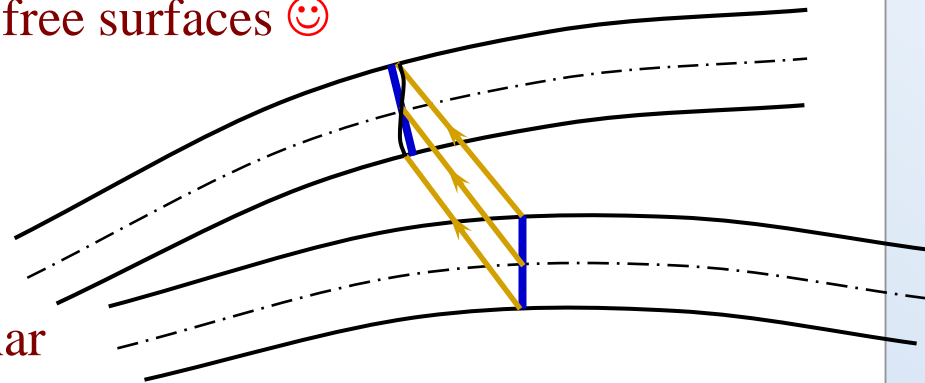
like before, only $g(z)$ is modified

⇒ planar sections do not remain planar

$$u_1(x, y, z) = u(x, y) + z \cdot \theta_2 + g(z) \cdot \gamma_5$$

$$u_2(x, y, z) = v(x, y) - z \cdot \theta_1 + g(z) \cdot \gamma_4$$

$$u_3(x, y, z) = w(x, y)$$



Remark: Thick shells:

most often: continuum models,

e.g. FEM: 3D elements (one layer or multi-layered)

masonry vaults: Discrete Element Method

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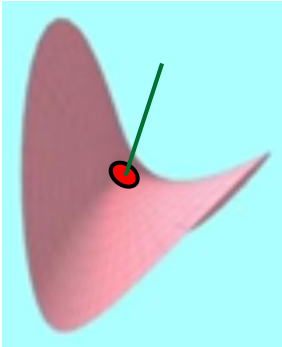
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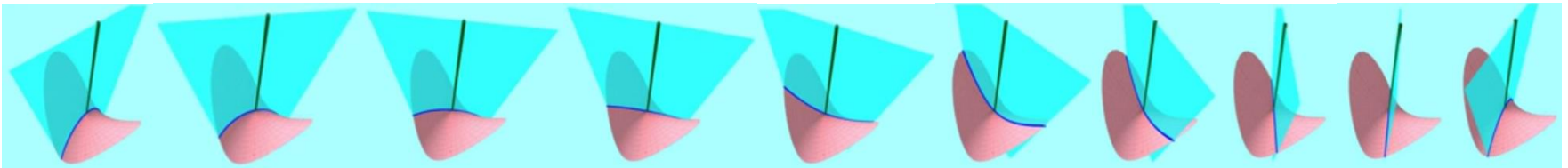
Questions

Repetition from Maths: Principal curvatures



Principal curvatures at a point of a surface:

- draw a straight axis;
- lay a plane along this axis \Rightarrow intersection along a curve;
- rotate the plane and produce these curves:



<https://www.youtube.com/watch?v=HUzOPbZk8Pg>

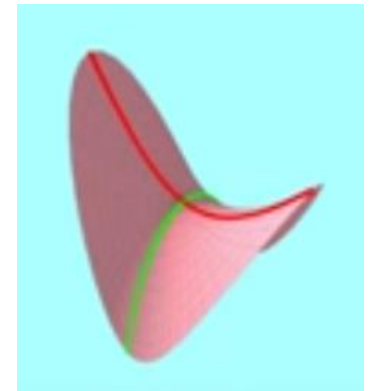
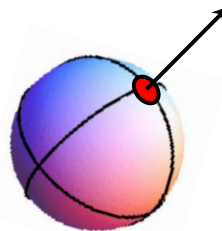
- take that two curves having largest / smallest curvature:

radii of curvature at the point:

R_{min} and R_{max} are received

for any point on a sphere:

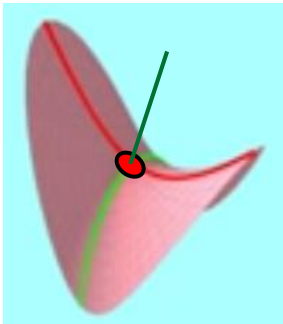
$$R_{min} = R_{max} = R$$



<https://slideplayer.com/slide/8958879/>

Repetition from Maths: Principal curvatures

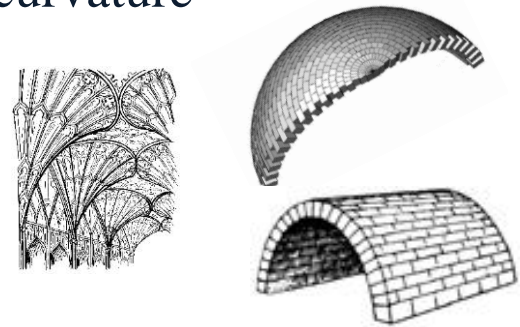
Product of the two principal curvatures: „Gaussian curvature”



→ if it is (+) : „elliptical point”

→ if it is (−) : „hyperbolic point”

→ if it is 0 : „parabolic point”



Its importance in the mechanical behaviour:

„... shells of nonpositive Gaussian curvature are generally speaking weak structures, sensitive to disturbances at the boundary, which tend to penetrate deep into the structure. Also, they have no possibility of supporting concentrated forces in the membrane state, in contrast with shells of positive Gaussian curvature, where the membrane state will provide an approximately correct result at sufficient distance from the point where a concentrated force is applied.” (Niordson, 1985)

„a local problem propagates to global collapse”

<https://www.youtube.com/watch?v=vAavRx7uoeA> Ph. Block, from 13:21 23 / 40

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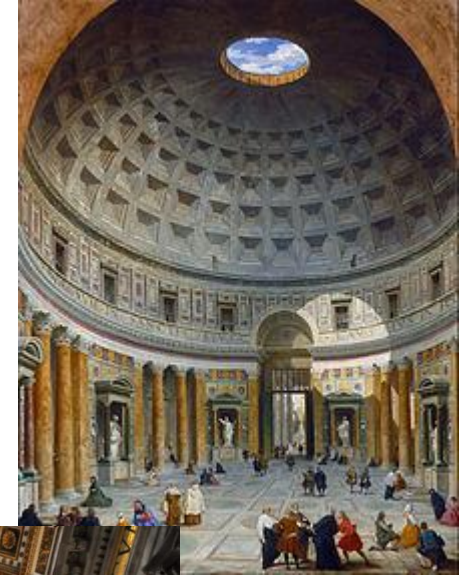
Membrane solution for fan vaults

Questions

Membrane solution for spherical domes



Hagia Sophia



Pantheon



St Peter's Basilica

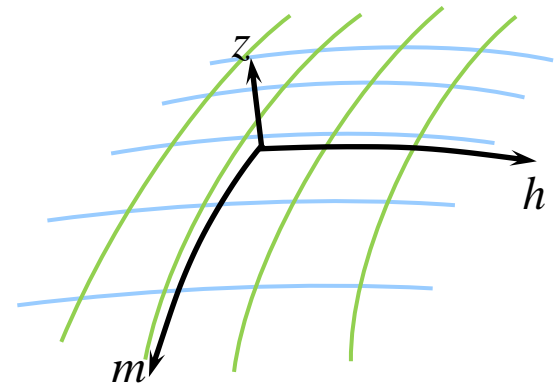
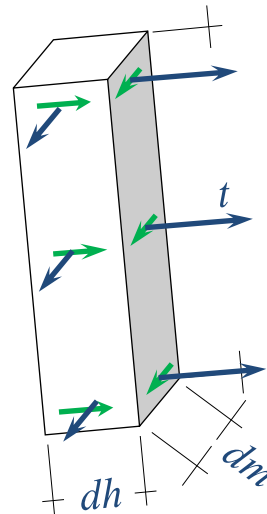
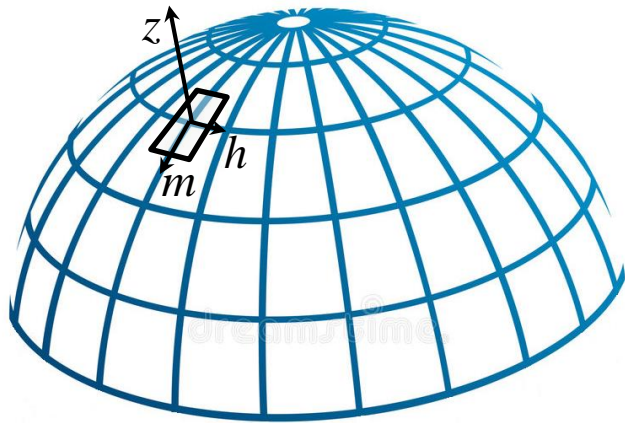
Membrane solution for spherical domes

Notations:

m : meridional direction

h : hoop direction

z : normal direction



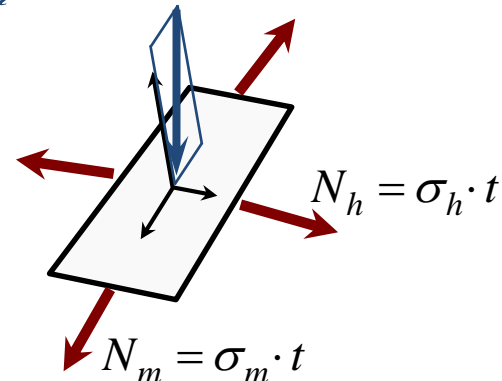
→ load: selfweight, only in vertical direction

→ stresses:

along t : σ_m , σ_h are constant

σ_z , τ_{mh} , τ_{mz} and τ_{hz} are zero

q : weight for unit area



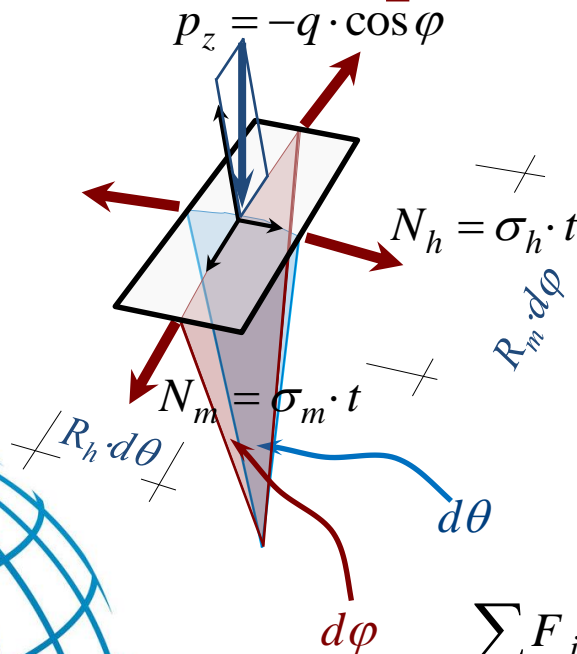
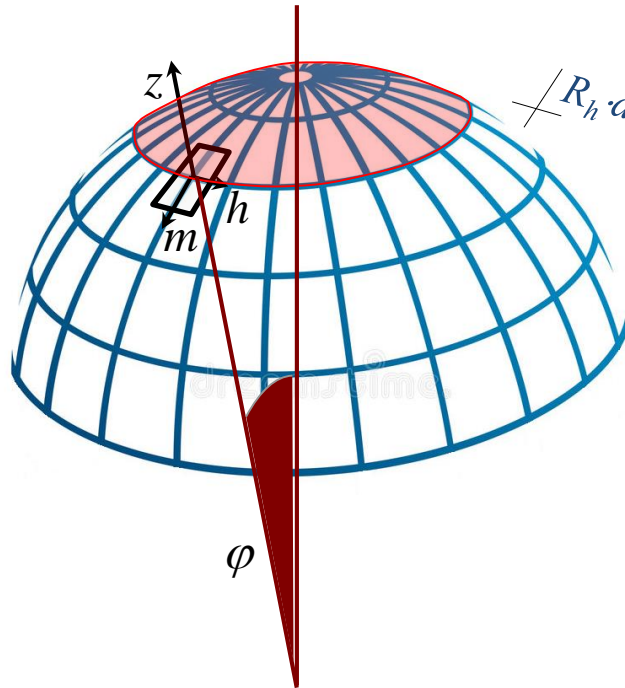
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$$\sum F_{iz} : \quad [\text{in normal dir.}]$$

$$2 \left(N_h \sin \frac{d\theta}{2} \right) \cdot R_m d\varphi +$$

$$+ 2 \left(N_m \sin \frac{d\varphi}{2} \right) \cdot R_h d\theta +$$

$$+ p_z \cdot R_m d\varphi \cdot R_h d\theta = 0$$

$$\frac{N_h}{R_h} + \frac{N_m}{R_m} = -q \cdot \cos \varphi$$

$\sum F_{i, \text{vertical}}$ for the cap:

$$(N_m \sin \varphi) \cdot 2\pi (R_m \cdot \sin \varphi) +$$

$$+ q \cdot (A_{\text{cap}}) = 0 \quad \leftarrow \text{for sphere:}$$

$$A_{\text{cap}} = 2\pi R^2 (1 - \cos \varphi)$$

$$N_m = -\frac{q \cdot R_m (1 - \cos \varphi)}{\sin^2 \varphi} = -\frac{q \cdot R_m (1 - \cos \varphi)}{1 - \cos^2 \varphi}$$

$$N_h = -R_h \cdot q \left(\cos \varphi - \frac{1}{(1 + \cos \varphi)} \right)$$

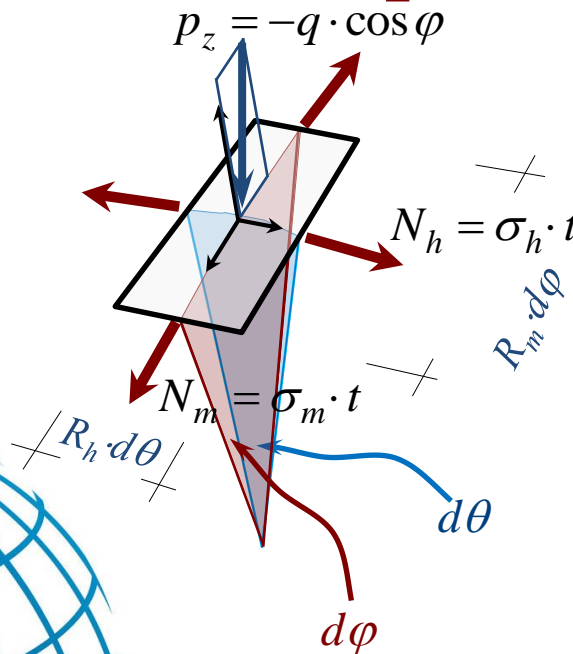
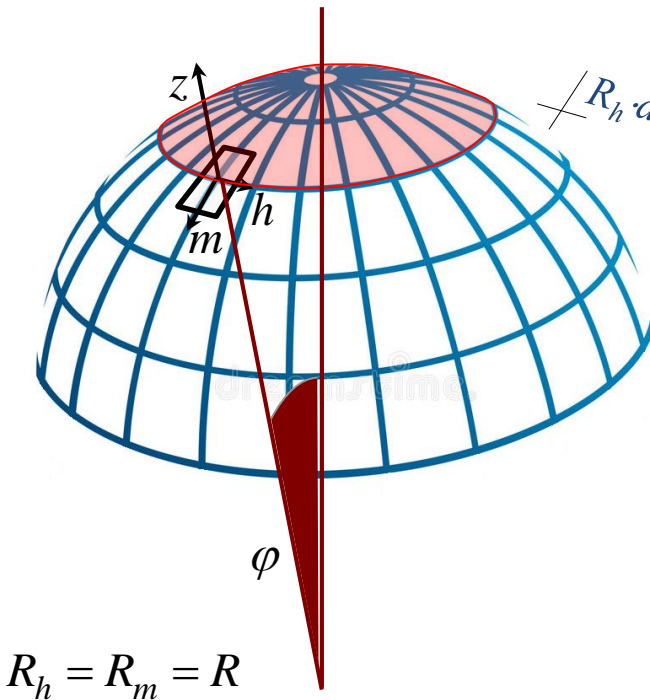
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$$\sigma_m = \frac{N_m}{t} = -\frac{q}{t} R \frac{1}{1 + \cos \varphi}$$

$$\sigma_h = \frac{N_h}{t} = -\frac{q}{t} R \left(\cos \varphi - \frac{1}{(1 + \cos \varphi)} \right)$$

$$N_m = -\frac{q \cdot R_m (1 - \cos \varphi)}{\sin^2 \varphi} = -\frac{q \cdot R_m (1 - \cos \varphi)}{1 - \cos^2 \varphi}$$

$$N_h = -R_h \cdot q \left(\cos \varphi - \frac{1}{(1 + \cos \varphi)} \right)$$

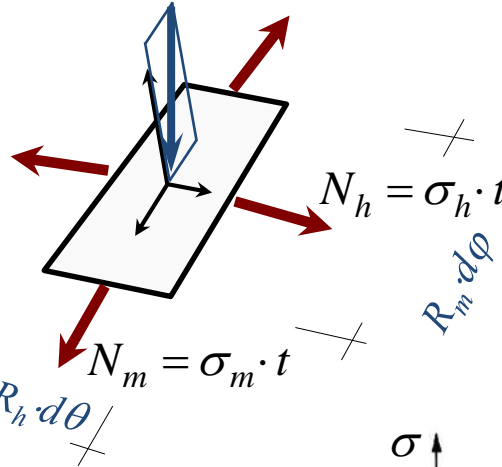
Membrane solution for spherical domes

Notations:

m : meridional direction

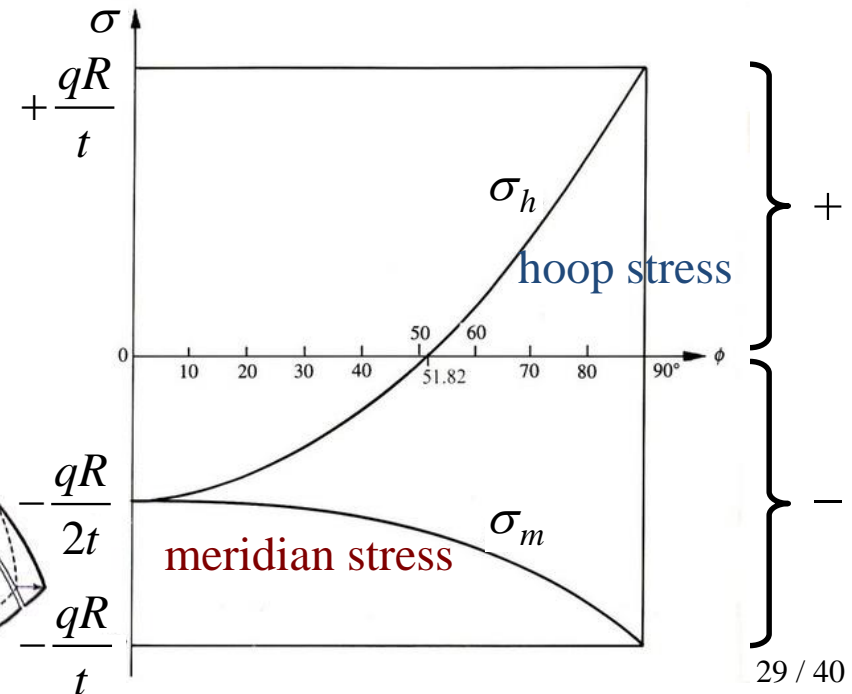
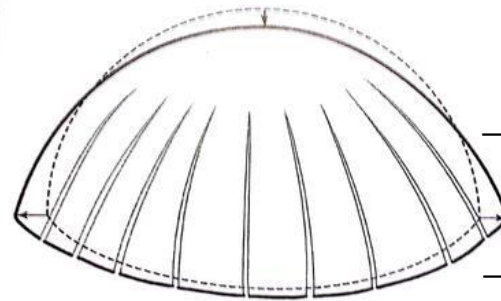
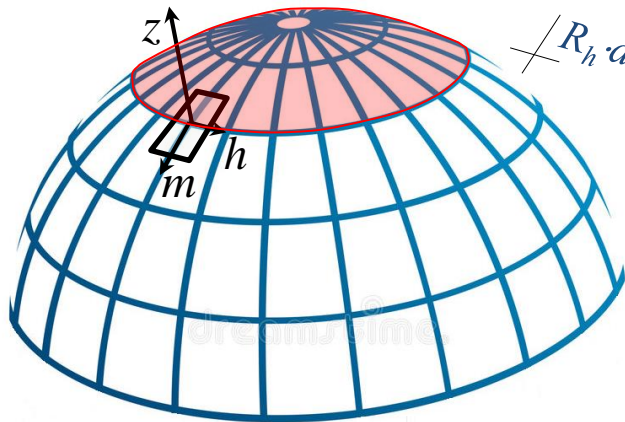
h : hoop direction

z : normal direction



$$\sigma_m = \frac{N_m}{t} = -\frac{q}{t} R \frac{1}{1 + \cos \varphi}$$

$$\sigma_h = \frac{N_h}{t} = -\frac{q}{t} R \left(\cos \varphi - \frac{1}{(1 + \cos \varphi)} \right)$$



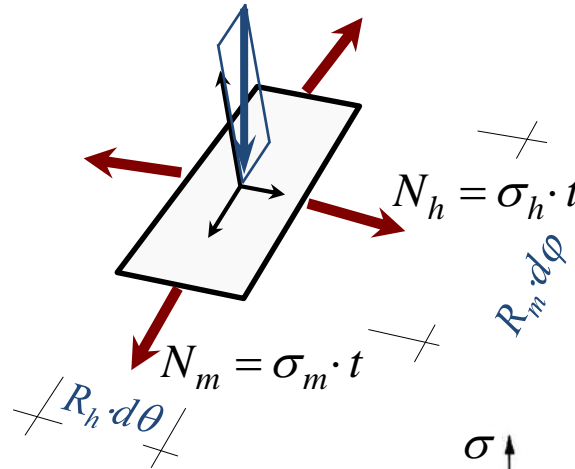
Membrane solution for spherical domes

Notations:

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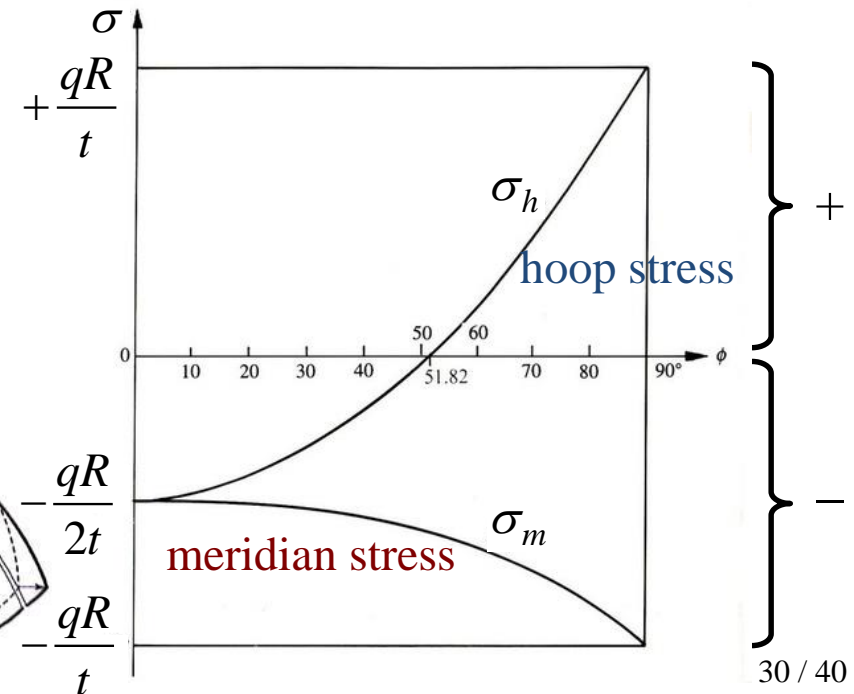
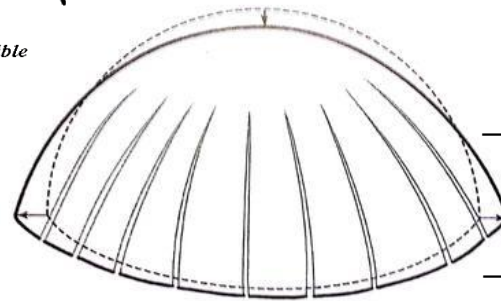
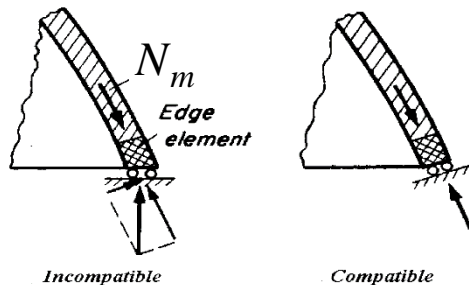
z : normal direction



$$\sigma_m = \frac{N_m}{t} = -\frac{q}{t} R \frac{1}{1 + \cos \varphi}$$

$$\sigma_h = \frac{N_h}{t} = -\frac{q}{t} R \left(\cos \varphi - \frac{1}{(1 + \cos \varphi)} \right)$$

Importance of boundaries:



THIS LECTURE:

SHELL THEORIES

Definition of shells and the shell element

The most important shell theories

Membranes; soap films

Kirchhoff-Love shells („classical shells”, „shell bending”)

Reissner-Mindlin shells („linear shear deformation theory”)

Third-order shear deformation theory

Membrane solution: Examples

Repetition from Maths: Principal curvatures at a surface point

Membrane solution for spherical domes

Membrane solution for fan vaults

Questions

Membrane solution for fan vaults



Bath Abbey, quillcards.com/blog/bath-abbey/



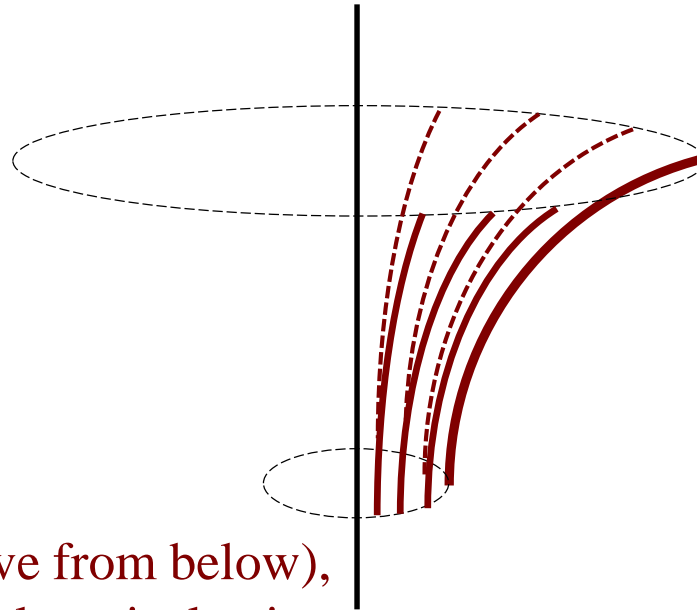
*Gloucester Cathedral
cloister walk,
slideshare.net/michaelasanda/gloucester-cathedral2*



King's College Chapel, quora.com/Are-there-any-buildings-with-fanned-vaulting-outside-of-the-US-UK-and-Canada

Membrane solution for fan vaults

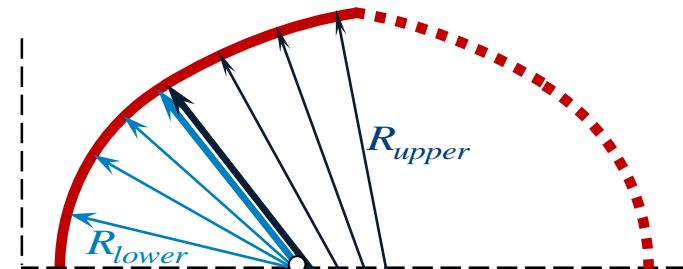
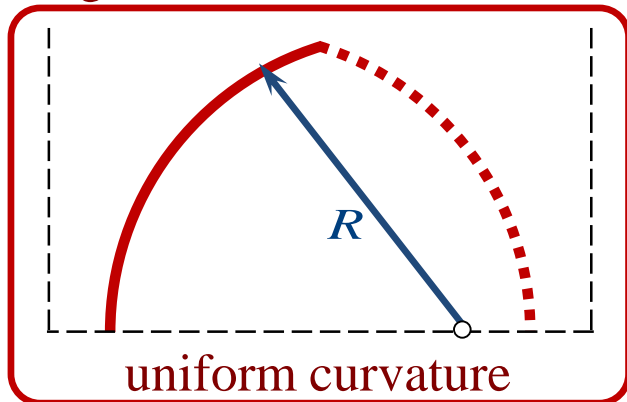
What is a fan vault?



Middle surface:

- generator curve (concave from below),
- rotated about an external vertical axis

The generator curve:



two different curvatures („Tudor arch”)

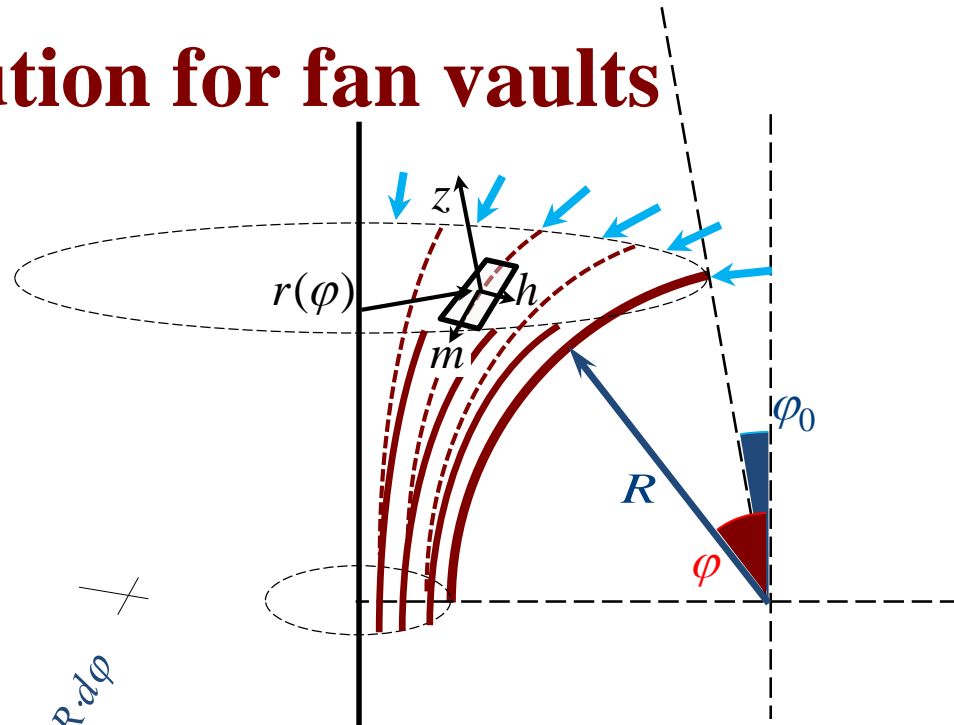
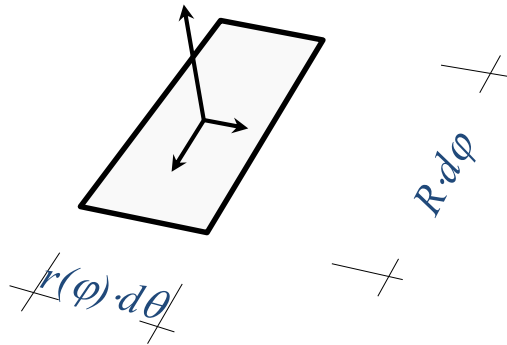
Membrane solution for fan vaults

Notations:

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z : normal direction

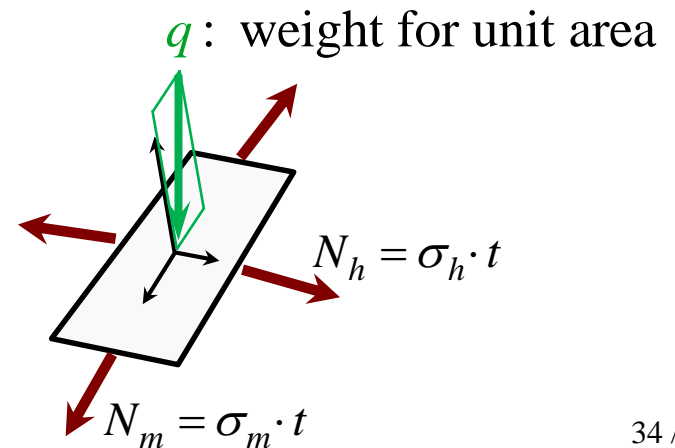


→ load: selfweight, vertical direction
spandrel load, meridional direction

→ stresses:

along t : σ_m , σ_h are constant

σ_z , τ_{mh} , τ_{mz} and τ_{hz} are zero



Membrane solution for fan vaults

Notations:

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h : hoop direction

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$$\sum F_{iz} : \frac{N_h}{r(\varphi)} \sin \varphi = \frac{N_m}{R} + q \cdot \cos \varphi$$

From meridional forces:

$$-(N_m \cdot r(\varphi) d\theta) \cdot d\varphi$$

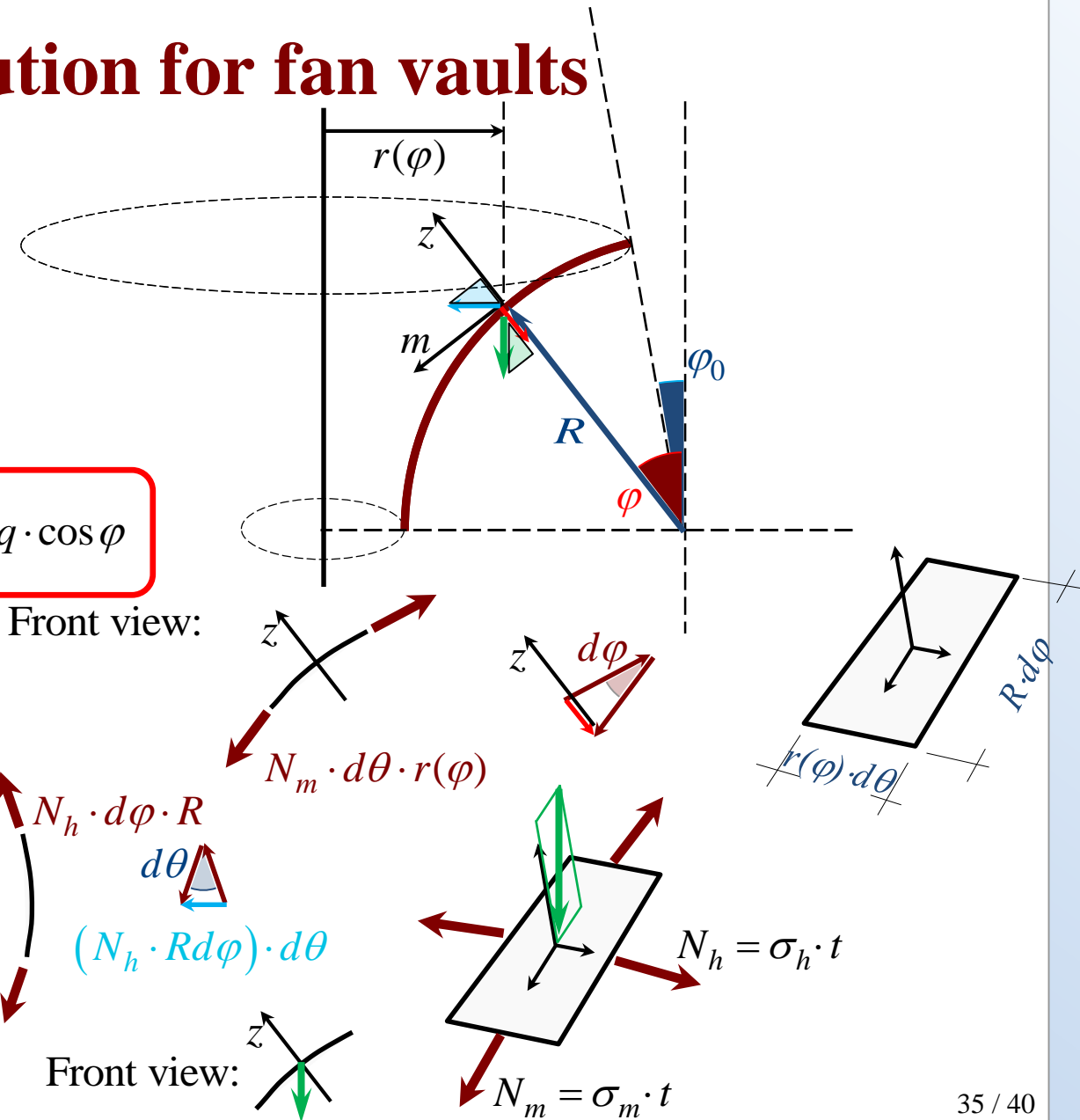
From hoop forces:

$$+(N_h \cdot R d\varphi) \cdot d\theta \cdot \sin \varphi$$

Top view:

From selfweight:

$$-(q \cdot R d\varphi \cdot r(\varphi) d\theta) \cdot \cos \varphi$$



Membrane solution for fan vaults

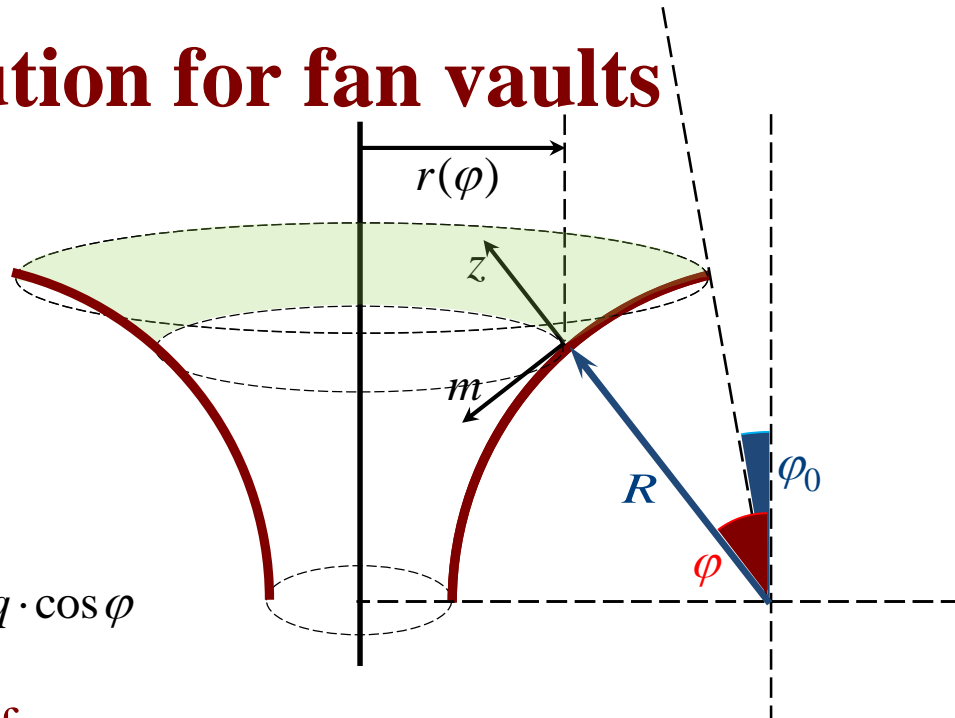
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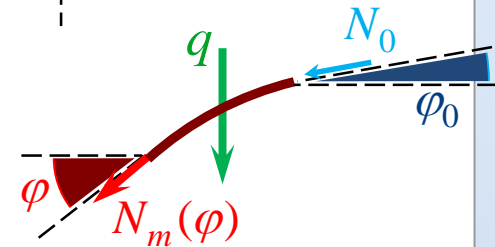
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$$\sum F_{iz} : \quad \frac{N_h}{r(\varphi)} \sin \varphi = \frac{N_m}{R} + q \cdot \cos \varphi$$



Determine the meridional force:

$$\sum F_{i,vertical} : \quad \left\{ \begin{array}{l} \text{Weight of the cap above } \varphi: A_{cap}(\varphi) \cdot q \\ \text{Membrane load at top, at } \varphi_0: N_0 \cdot \sin \varphi_0 \cdot \pi \cdot r(\varphi_0) \\ \text{Meridional force at } \varphi: N_m(\varphi) \cdot \sin \varphi \cdot \pi \cdot r(\varphi) \end{array} \right.$$



$$N_m(\varphi) \cdot \sin \varphi \cdot \pi \cdot r(\varphi) + N_0 \cdot \sin \varphi_0 \cdot \pi \cdot r(\varphi_0) + A_{cap}(\varphi) \cdot q = 0$$

$$N_m(\varphi) = \frac{-1}{\sin \varphi \cdot r(\varphi)} \left(N_0 \cdot \sin \varphi_0 \cdot r(\varphi_0) + \frac{A_{cap}(\varphi)}{\pi} \cdot q \right) \Rightarrow N_h(\varphi) = \frac{r(\varphi)}{\sin \varphi} \left(\frac{N_m(\varphi)}{R} + q \cdot \cos \varphi \right)$$

Membrane solution for fan vaults

Remarks:

1) if $\varphi_0 = 0$:

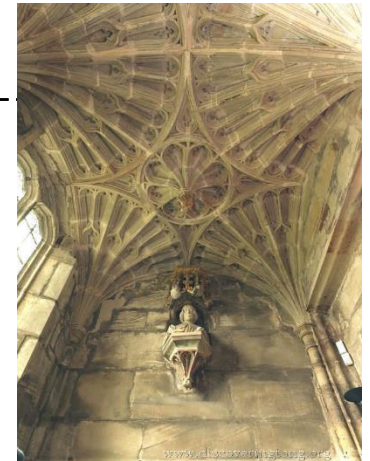
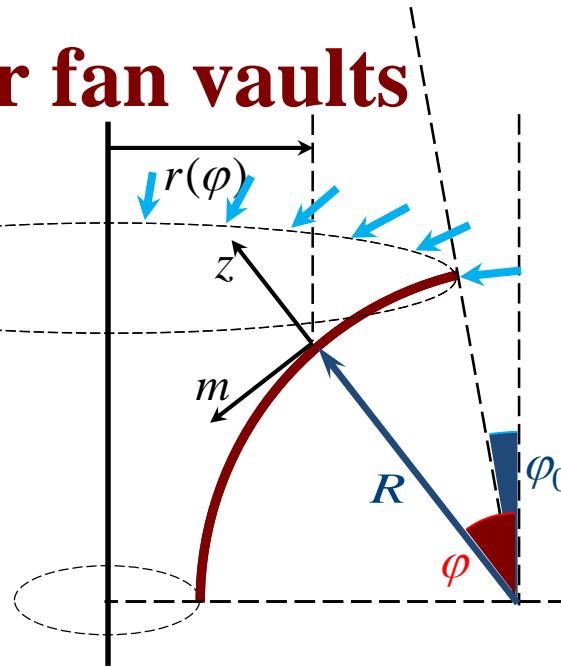
$$\sin \varphi_0 = 0 \Rightarrow N_h(\varphi_0) \rightarrow \infty$$

\Rightarrow the conoid must be **truncated** !

2) if $\varphi_0 \neq 0$:

$$\text{if } N_0 = 0 : N_m(\varphi_0) = 0 \Rightarrow N_h > 0$$

\Rightarrow **spandrel load** is needed to avoid hoop tension !



Tong Church Chapel,
<http://www.discoveringtong.org/tong600/ChapelRoof.htm>

$$N_m(\varphi) = \frac{-1}{\sin \varphi \cdot r(\varphi)} \left(\boxed{N_0} \cdot \sin \varphi_0 \cdot r(\varphi_0) + \frac{A_{cap}(\varphi)}{\pi} \cdot q \right)$$

$$N_h(\varphi) = \frac{r(\varphi)}{\boxed{\sin \varphi}} \left(\frac{N_m(\varphi)}{R} + q \cdot \cos \varphi \right)$$

SUGGESTED VIDEOS

<https://www.youtube.com/watch?v=DI-leSI68dM> (Jacques Heyman: The membrane analysis of thin masonry shells, 50:46)

<https://www.youtube.com/watch?v=r-tG68WvNDM&t=185s>
(John Ochsendorf, MIT, „Form and Forces”, 1:17:17)

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- Third-order shear deformation theory

Membrane solution:

- Repetition from Maths: Principal curvatures at a surface point

- Membrane solution for spherical domes

- Membrane solution for fan vaults

Questions

Questions

1. Introduce the **kinematics** of
 - membranes,
 - Kirchhoff-Love shells,
 - Mindlin-Reissner-Hencky shells,
 - third-order shear deformation shells.
2. Show the distribution of the **six stress components** along the thickness for
 - membranes,
 - Kirchhoff-Love shells,
 - Mindlin-Reissner-Hencky shells,
 - third-order shear deformation shells.
3. Introduce the membrane solution for **spherical domes** under selfweight.
4. Introduce the membrane solution for **constant-curvature fan vaults** under selfweight.