

# DDA



#### **OVERVIEW OF DEM SOFTWARES**

<u>Quasi-static methods</u>  $\leftarrow$  <u>equilibrium states</u> are searched for From an initial approximation of the equilibrium state searched for,

the displacements **u** are to be determined taking the system to the equilibrium (assumption: time-independent behaviour, zero accelerations!!!)

 $\mathbf{W}\mathbf{K}\cdot\Delta\mathbf{u}+\mathbf{f}=\mathbf{0}\mathbf{W}$ 

<u>Time-stepping methods</u> " $\mathbf{M} \cdot \mathbf{a}(t) = \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t))$ "  $\leftarrow a \text{ process in time}$  is searched for

simulate the motion of the system along small, but finite  $\Delta t$  timesteps

Explicit timestepping methods:

 $\rightarrow$  Polyhedral elements, e.g. UDEC *rigid / deformable elements; deformable contacts* 

 $\rightarrow$  BALL-type models, e.g. PFC rigid elements; deformable contacts Implicit timestepping methods:

→ DDA (,,Discontinuous Deformation Analysis") deformable polyhedral elements → Contact Dynamics models rigid elements, non-deformable contacts

"DDA": Gen-Hua Shi (1988), Berkeley then many others applied or developed research softwares!!!



#### 1. The elements: The unknowns and the reduced loads

2. Contacts

- 3. The equations of motion: "Potential energy minimization"
- 4. Numerical solution of the equations of motion
- 5. Comparison to 3DEC

Applications

Questions





How to reduce a force acting at (x, y) to the reference point of the element:

 $\mathbf{f}_{x}^{f}(x,y) = \begin{bmatrix} f_{x}^{p} \\ f_{y}^{p} \\ m_{z}^{p} \\ A^{p}\sigma_{y}^{p} \\ A^{p}\sigma_{y}^{p} \\ A^{p}\tau_{xy}^{p} \end{bmatrix} \leftarrow \begin{bmatrix} 1 & 0 \\ 0 & 1 \\ -(y-y^{p}) & (x-x^{p}) \\ (x-x^{p}) & 0 \\ 0 & (y-y^{p}) \\ (\underline{y-y^{p}}) & (\underline{x-x^{p}}) \\ \frac{(y-y^{p})}{2} & \frac{(x-x^{p})}{2} \end{bmatrix} \begin{bmatrix} f_{x}(x,y) \\ f_{y}(x,y) \end{bmatrix} =$ e.g. in 2D: Illustration of its meaning:

 $= \mathbf{T}^{T}(x, y) \cdot \mathbf{f}(x, y)$ 

This should be collected for every force that acts on the element, and then summed up. 6/32











How to reduce a force acting at (x, y) to the reference point of the element:



the reduced force in 3D:  $\mathbf{T}^{T}(x, y, z) \cdot \mathbf{f}(x, y, z)$ 

<u>Remark:</u> Higher-order polynomials also possible! [strains/stresses] (e.g: M. MacLaughlin)

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 $\rightarrow$  recent codes: deformable contacts included

+ other friction conditions, cohesion etc.

<u>Remark:</u> infinitely rigid contact: "penalty function":  $F_N = k_N \Delta u_N$ ;  $dF_T = k_T d(\Delta u_T)$ = linearly elastic in normal and in tangential directions<sub>14/32</sub>

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 $\downarrow$  more exactly: "Hamilton principle"

3. The equations of motion: "Potential energy" stationarity principle



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4. Numerical solution of the equations of motion:

 $(\underline{t}_i, \underline{t}_{i+1})$  time interval: at  $t_i$ : known  $\mathbf{u}_i$ ,  $\mathbf{v}_i$ ,  $\mathbf{f}(t_i, \mathbf{u}_i, \mathbf{v}_i)$ ; satisfy the eqs. of motion Find  $\mathbf{u}_{i+1}$ ,  $\mathbf{v}_{i+1}$ ,  $\mathbf{a}_{i+1}$  so that the eqs of motion would be satisfied at  $t_{i+1}$  $\mathbf{r}(t_{i+1}, \mathbf{u}_{i+1}, \mathbf{v}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}, \mathbf{v}_{i+1}) - \mathbf{M} \cdot \mathbf{a}_{i+1} = 0$ Newmark's  $\beta$ -method:  $\mathbf{u}_{i+1} = \mathbf{u}_i + \Delta t \cdot \mathbf{v}_i + \frac{\Delta t^2}{2} \left[ (1 - 2\beta) \mathbf{a}_i + 2\beta \cdot \mathbf{a}_{i+1} \right]$   $\mathbf{v}_{i+1} \coloneqq \mathbf{v}_i + (1 - \gamma) \cdot \Delta t \cdot \mathbf{a}_i + \gamma \cdot \Delta t \cdot \mathbf{a}_{i+1}$ Remember: [stability:  $2\beta \ge \gamma \ge \frac{1}{2}$ ] Newmark's  $\beta$ -method, with  $\beta = 1/2$ ;  $\gamma = 1$ :  $\mathbf{u}_{i+1} = \mathbf{u}_i + \Delta t \cdot \mathbf{v}_i + \frac{\Delta t^2}{2} \mathbf{a}_{i+1}$   $\mathbf{v}_{i+1} \coloneqq \mathbf{v}_i + \Delta t \cdot \mathbf{a}_{i+1}$ DDA:

> Mechanical meaning of this choice of  $\beta$  and  $\gamma$ : The acceleration that will be valid at the **end of the timestep** is considered valid throughout **the complete timestep**

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4. Numerical solution of the equations of motion:

 $(\underline{t}_i, \underline{t}_{i+1})$  time interval: at  $t_i$ : known  $\mathbf{u}_i$ ,  $\mathbf{v}_i$ ,  $\mathbf{f}(t_i, \mathbf{u}_i, \mathbf{v}_i)$ ; satisfy the eqs. of motion Find  $\mathbf{u}_{i+1}$ ,  $\mathbf{v}_{i+1}$ ,  $\mathbf{a}_{i+1}$  so that the eqs of motion would be satisfied at  $t_{i+1}$  $\mathbf{r}(t_{i+1}, \mathbf{u}_{i+1}, \mathbf{v}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}, \mathbf{v}_{i+1}) - \mathbf{M} \cdot \mathbf{a}_{i+1} = 0$ Newmark's  $\beta$ -method:  $\mathbf{u}_{i+1} = \mathbf{u}_i + \Delta t \cdot \mathbf{v}_i + \frac{\Delta t^2}{2} [(1-2\beta)\mathbf{a}_i + 2\beta \cdot \mathbf{a}_{i+1}]$   $\mathbf{v}_{i+1} \coloneqq \mathbf{v}_i + (1-\gamma) \cdot \Delta t \cdot \mathbf{a}_i + \gamma \cdot \Delta t \cdot \mathbf{a}_{i+1}$ Remember: Newmark's  $\beta$ -method, with  $\beta = 1/2$ ;  $\gamma = 1$ :  $\mathbf{u}_{i+1} = \mathbf{u}_i + \Delta t \cdot \mathbf{v}_i + \frac{\Delta t^2}{2} \mathbf{a}_{i+1}$   $\mathbf{v}_{i+1} = \mathbf{v}_i + \Delta t \cdot \mathbf{a}_{i+1}$   $\mathbf{v}_{i+1} = \mathbf{v}_i + \Delta t \cdot \mathbf{v}_i$   $\mathbf{v}_{i+1} = \mathbf{v}_i + \Delta t \cdot \mathbf{a}_{i+1} = \mathbf{v}_i + \frac{2}{\Delta t} (\Delta \mathbf{u}_{i+1} - \Delta t \cdot \mathbf{v}_i) = \frac{2}{\Delta t} \Delta \mathbf{u}_{i+1} - \mathbf{v}_i$ 19/32 DDA:

4. Numerical solution of the equations of motion :

 $\mathbf{M} \cdot \mathbf{a}(t) = \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t)) \implies \mathbf{0} = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}, \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$ 

Determine  $\Delta \mathbf{u}_{i+1}$  so that the residual  $\mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}), \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$ would be sufficiently close to zero!

Newton-Raphson:

the Jacobian of the residual:  $\mathcal{K}(t, \Delta \mathbf{u}) = \frac{d\mathbf{r}(t, \Delta \mathbf{u})}{d\Delta \mathbf{u}}$ 

this matrix can be compiled from elementary calculations at t<sub>i</sub>:
← contains the stiffness matrix
← contains the inertia, contact forces, geometric characteristics etc.

the residual can also be compiled from elementary calculations at  $t_i$ :  $\leftarrow$  contains the external forces, inertia effects, prescribed displacements, damping etc.

4. Numerical solution of the equations of motion :

 $\mathbf{M} \cdot \mathbf{a}(t) = \mathbf{f}(t, \mathbf{u}(t), \mathbf{v}(t)) \implies \mathbf{0} = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}, \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$  $\mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}) = \mathbf{f}(t_{i+1}, \mathbf{u}_{i+1}(\Delta \mathbf{u}_{i+1}), \mathbf{v}_{i+1}(\Delta \mathbf{u}_{i+1})) - \mathbf{M} \cdot \mathbf{a}_{i+1}(\Delta \mathbf{u}_{i+1})$  $\mathcal{K}(t,\Delta \mathbf{u}) = \frac{d\mathbf{r}(t,\Delta \mathbf{u})}{d\mathbf{r}(t,\Delta \mathbf{u})}$ Analysis of a time interval: initial estimation for  $\Delta \mathbf{u}_{i+1}$ :  $\Delta \mathbf{u}_{i+1}^{(0)} \coloneqq \mathbf{0}$ *k*+1-th estimation for  $\Delta \mathbf{u}_{i+1}$ :  $\Delta \mathbf{u}_{i+1}^{(k+1)} \coloneqq \Delta \mathbf{u}_{i+1}^{(k)} - \mathcal{K}(t_{i+1}, \Delta \mathbf{u}_{i+1}^{(k)})^{-1} \cdot \mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}^{(k)})$ then continue until  $|\mathbf{r}(t_{i+1}, \Delta \mathbf{u}_{i+1}^{k+1})|$  becomes sufficiently small "Open – close iterations": at the end of  $\Delta t$ : check the topology and the forces; → modify the topology if necessary (e.g. new contacts, sliding, contact loss)  $\rightarrow$  with the new topology, **repeat:** Newton-Raphson to find another  $\Delta \mathbf{u}_{i+1}$ if acceptable topology not found: decrease timestep  $\Delta t$  to 1/3 of its previous length **CONVERGENCE WITHIN A TIME STEP ???** 21/32

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5. Comparison to 3DEC:

#### Main differences from 3DEC:

- $\rightarrow$  basic unknowns: also the components of  $\epsilon$ ;
- $\rightarrow$  uniform stress and strain field inside the elements;
- $\rightarrow$  numerical integration: implicit
- $\rightarrow$  stiffness matrix included  $\Rightarrow$  artificial damping not necessary
- <u>advantages to 3DEC:</u>  $\rightarrow$  implicit  $\Rightarrow$  numerical stability;

fast convergence if topology does not change no artificial damping required

 <u>disadvantages:</u> no commercial software ⇒ inconvenient (several research codes; e.g. ask from Gen-Hua Shi) too simple mechanics of the elements and of the contacts large storage requirements & longer computations open-close iterations: convergence is not ensured if topology changes



SIGNIFICANT TOPOLOGY MODIFICTIONS OCCUR !!!

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#### **Applications**



#### arches, arch bridges



### underground excavations

#### masonry structures

e.g. Fractured rock for earthquake and thermal effects

230 m



#### **DISCONTINUOUS DEFORMATION**

Y.H.Hatzor, 1999-2004: Masada-hill, Israel (King Herod's Palace)



King Herod the Great built his fortress a hundred kilometers away from Jerusalem, partly as a retreat, but more as a place of refuge for himself.

It happened at Masada seventy years after Herod's death that the Jewish Revolt against the Roman occupation was well underway. As the story goes, a last brave group of Jewish rebels fled Jerusalem to Masada, and withstood a two-year long siege by a 15,000-strong Roman Army. The crafty Romans finally built a ramp up the mountain slope.

On their last night in Masada before the Romans broke through, the 967 rebels opted for mass suicide rather than surrender.

"Masada must not fall again"







Y.H.Hatzor, 1999-2004: Masada-hill, Israel (King Herod's Palace) <u>experience:</u> huge blocks released, or already fallen out

the aim of the analysis: is it stable for an earthquake? "Masada must not fall again"

– DDA model, earthquake simulation;





Y.H.Hatzor, 1999-2004:

Masada-hill, Israel (King Herod's Palace)

experience: huge blocks released, or already fallen out

the aim of the analysis: is it stable for an earthquake? "Masada must not fall again"

- DDA model, earthquake simulation;
- minor earthquake: considerable damages can be expected;
- 7,1 earthquake (1995, Sinai-desert): huge blocks would fall out

 $\Rightarrow$  proposals for strengthening





#### **DISCONTINUOUS DEFORMATION ANALYSIS** East Mazor (2011): 3D investigations 1 m EJM 1 -----Wedge Block ake Path" Rock EJM 2 Sliding Mass Block Sliding Surface $(\phi)$ Tension Crack L \_\_\_\_ $\rightarrow$ $L_{\rm w} \leftarrow L_{\rm B} = 7.5 \,\mathrm{m} \longrightarrow$ initial condition cooling heating cycle 1 Block 1 H= 15.0 m Wedges cycle 2 η = 19° Fixed rock mass

#### QUESTIONS

1. How the displacement vector of an element looks like in DDA? (i.e., What are the degrees of freedom of an element in DDA?)

2. What are the characteristic contact deformations in DDA? How are the contact forces calculated from them? What is the mechanical meaning of the "penalty function"?

3. What kind of time integration method is used in DDA? What values of those numerical control parameters  $\beta$  and  $\gamma$  are applied in it, and what is the mechanical meaning of this choice?

4. Explain the meaning of the expression "open-close iteration" and how it is used in DDA.

5. Tell five differences between DDA and 3DEC.